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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-1
Field Sample Number:	TP 1	Depth (m):	1.7

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 1

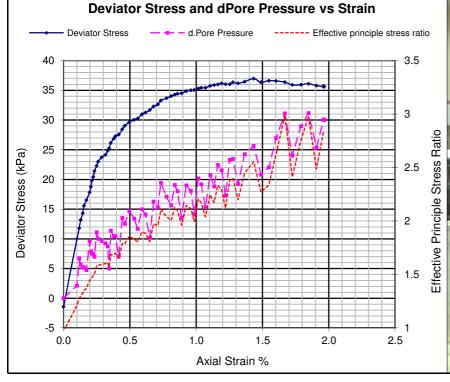
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 353.0	Final back pressure (kPa): 343.0	Final B parameter: 1.00

CONSOLIDATION DATA

Effective cons. Stress (kPa): 49.4		t100 (minutes): 9			Side drains fitted: Yes			
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity
INITIAL (Before saturation)	*100	*50	1963.50	2.0	1591	0.6554	8	0.000
CONSOLIDATED	99.55	49.77	1945.68	12.5	1612	0.6329	52	2.633 Determined
FINAL (After shear)	97.59	50.27	1984.61	12.5	1612	0.6330	52	Determined
Initial pore pressure (kPa): 383.7 Fin			al pore pre	ssure (kPa):	356.0	PWP dissip	ation (%): 10	00
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.22					
Initial pore pressure (kPa):	344.6	Initial effective stress (kPa):	49.4			
Parameters at failure:						
Failure Criterion:	Max. Deviator Stre	ess				
Axial strain (%):	1.43					
Deviator stress (kPa):	37.0		Principle Stresses (kPa)			
Excess pore pressure (kPa): 25.6 σ_1 σ_3				G 3'		
Effective principle stress rat	tio: 2.554		86.4	60.8	49.4	23.8





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Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-1
Field Sample Number:	TP 1	Depth (m):	1.7

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 2

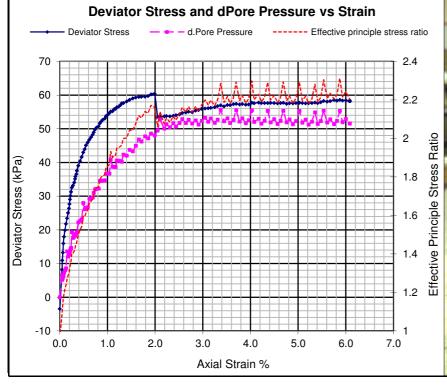
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 343.0	Final back pressure (kPa): 333.0	Final B parameter: 0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa):		100.0 t100 (minutes): 100			Side drains fitted: Yes			
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity
INITIAL (Before saturation)	*100	*50	1963.50	34.4	1220	1.1586	78	0.000
CONSOLIDATED	98.35	49.17	1898.81	45.6	1283	1.0519	114	2.633 Determined
FINAL (After shear)	92.37	50.74	2021.79	45.6	1282	1.0531	114	Determined
Initial pore pressure (kPa): 420.4 Fina			al pore pres	ssure (kPa):	335.7	PWP dissip	ation (%): 9	9
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.24					
Initial pore pressure (kPa):	336.0	Initial effective stress (kPa):	100.0			
Parameters at failure:						-
Failure Criterion:	Max. Deviator Str	ess				
Axial strain (%):	1.99					
Deviator stress (kPa):	60.4		Principle Stresses (kPa))
Excess pore pressure (kPa): 48.0			σ1	σ1'	O 3	O 3'
Effective principle stress rati	io: 2.162		160.4	112.3	100.0	52.0





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Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-1
Field Sample Number:	TP 1	Depth (m):	1.7

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 3

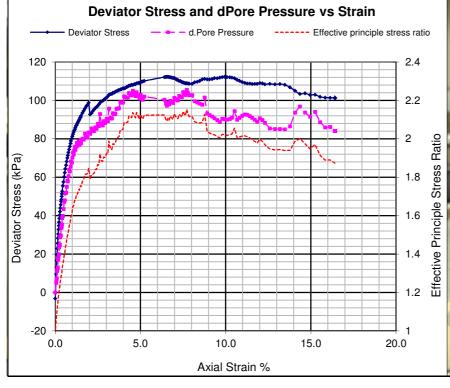
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 353.0	Final back pressure (kPa): 343.0	Final B parameter: 0.99

CONSOLIDATION DATA

Effective cons. Stress (kPa): 200.1		t100 (minutes): 400			Side drains fitted: Yes				
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific	
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity	
INITIAL (Before saturation)	*100	*50	1963.50	33.0	1243	1.1188	78	0.000	
CONSOLIDATED	97.10	48.53	1849.58	41.1	1361	0.9344	116	2.633 Determined	
FINAL (After shear)	81.16	53.08	2212.79	41.1	1359	0.9380	116	Determined	
Initial pore pressure (kPa): 506.1 Fina			al pore pres	ssure (kPa):	342.0	PWP dissip	ation (%): 10	00	
*: Measured dimensions; a	: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.12						
Initial pore pressure (kPa):	341.9	Initial effective stre	ess (kPa): 200	.1			
Parameters at failure:							
Failure Criterion:	Max. Effective Prir	nciple Stress Ratio					
Axial strain (%):	7.72						
Deviator stress (kPa):	108.9			Principle Stresses (kPa)			
Excess pore pressure (kPa): 105.5 σ_1 σ_3				G 3'			
Effective principle stress rate	tio: 2.152		3	309.0	203.5	200.1	94.6





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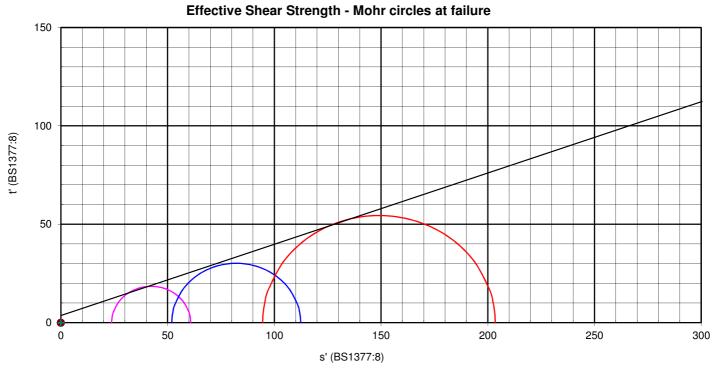
Civil Engineering Testing Laboratory

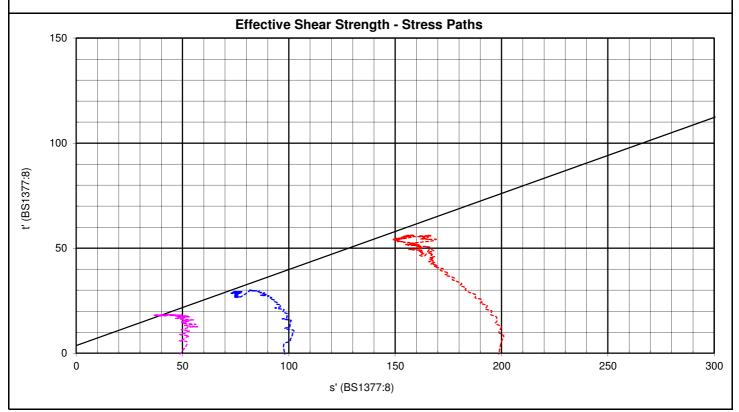
Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-1
Field Sample Reference:	TP 1	Depth (m):	1.7

Effective Shear Strength Parameters

Stresses	Cohesion (kPa)	Internal friction (Degrees)
Total	5.3	11.1
Effective	3.6	19.9





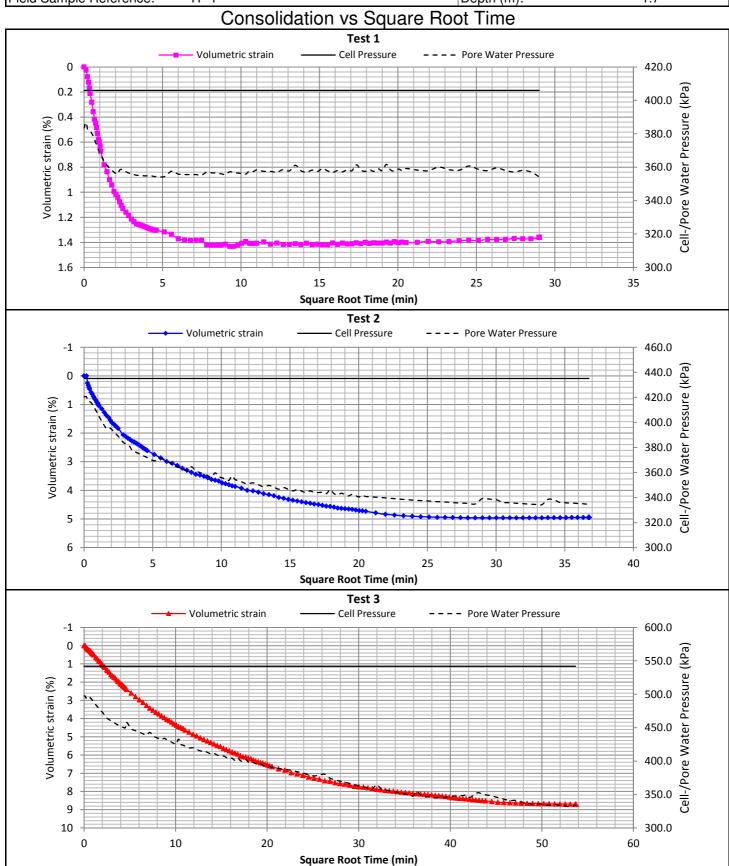
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Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-1
Field Sample Reference:	TP 1	Depth (m):	1.7



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Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-2
Field Sample Number:	TP 2	Depth (m):	1.8

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on an undisturbed sample tested saturated. The data from this test was not included in
	the determining the shear strength parameters (c' and phi). PWP pressure sensor suspected of being too unstable
	prior to being replaced. Insufficient time & sample to repeat specimen.

SATURATION DATA Test No. 1

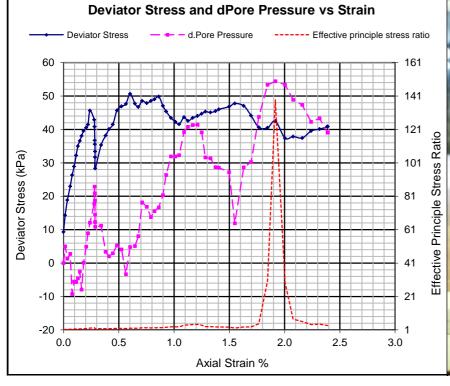
Saturation method: Alternating	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 313.0	Final back pressure (kPa): 303.0	Final B parameter: 0.96

CONSOLIDATION DATA

Effective cons. Stress (kPa): 54.7		t10	t100 (minutes): 144			Side drains fitted: Yes		
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity
INITIAL (Before saturation)	*100.9	*50.45	1999.00	28.9	1317	1.0732	74	0.704
CONSOLIDATED	100.58	50.29	1986.33	40.0	1330	1.0535	104	2.731 Determined
FINAL (After shear)	98.18	50.90	2034.94	40.0	1330	1.0535	104	Determined
Initial pore pressure (kPa):	339.1	Final pore pressure (kPa): 285.0			PWP dissip	ation (%): 10	00	
*: Measured dimensions; a	*: Measured dimensions; all other dimensions are calculated.							

SHEAR DATA

Rate of strain (%/hour):	0.045					
Initial pore pressure (kPa):	295.3	Initial effective stress (kPa):	54.7			
Parameters at failure:						
Failure Criterion:	Max. Deviator Stre	ess				
Axial strain (%):	0.60					
Deviator stress (kPa):	50.7			Principle Str	esses (kPa)	
Excess pore pressure (kPa)): 4.8		σ1	σ1'	σ3	σ3'
Effective principle stress rat	io: 2.015		105.4	100.6	54.7	49.9





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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-2
Field Sample Number:	TP 2	Depth (m):	1.8

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

Test No. 2 **SATURATION DATA**

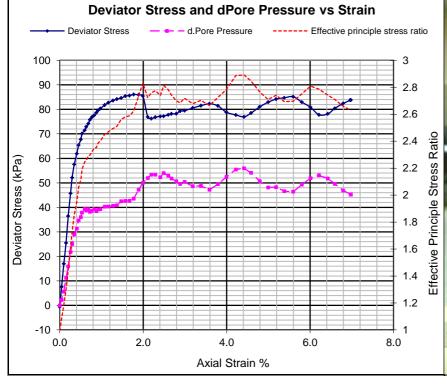
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 353.0	Final back pressure (kPa): 343.0	Final B parameter: 0.96

CONSOLIDATION DATA

Effective cons. Stress (kPa): 96.6		96.6	t100 (minutes): 400			Side drains fitted: Yes		
Height		Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity
INITIAL (Before saturation)	*101	*50.5	2002.96	29.5	1346	1.0297	78	0.704
CONSOLIDATED	100.41	50.20	1979.55	31.9	1370	0.9941	88	2.731 Determined
FINAL (After shear)	93.41	52.05	2127.99	31.9	1369	0.9942	87	Determined
Initial pore pressure (kPa): 433.6 Final pore pressure (kPa): 326.1 PWP dissipation (%): 100						00		
*: Measured dimensions; a	II other dim	ensions are	calculated.					

SHEAR DATA

Rate of strain (%/hour):	0.080					
Initial pore pressure (kPa):	347.4 I	nitial effective stress (kPa):	96.6			
Parameters at failure:						
Failure Criterion: M	lax. Deviator Stres	SS				
Axial strain (%):	1.77					
Deviator stress (kPa): 86.1 Principle Stresses (kPa))	
Excess pore pressure (kPa):	: 43.5		σ1	σ1'	σ3	σ3'
Effective principle stress ratio	o: 2.622		182.7	139.2	96.6	53.1





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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-2
Field Sample Number:	TP 2	Depth (m):	1.8

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 3

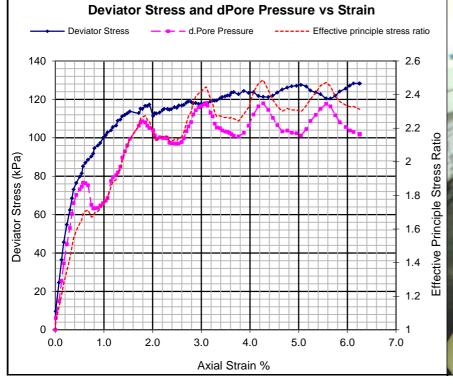
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 353.0	Final back pressure (kPa): 343.0	Final B parameter: 0.96

CONSOLIDATION DATA

Effective cons. Stress (kPa	199.5 t100 (minutes): 625			625	Side drains fitted: Yes					
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific		
	mm	mm	mm^2	Content %	kg/m ³	Ratio	%	Gravity		
INITIAL (Before saturation)	*101.8	*50.9	2034.82	30.1	1255	1.1758	70	0.704		
CONSOLIDATED	99.90	49.94	1958.89	38.6	1330	1.0540	100	2.731 Determined		
FINAL (After shear)	93.65	51.58	2089.53	38.6	1329	1.0556	100	Determined		
Initial pore pressure (kPa): 339.6 Final pore pressure (kPa): 338.9 PV							ation (%): 10	00		
*: Measured dimensions; all other dimensions are calculated.										

SHEAR DATA

Rate of strain (%/hour):	0.805					
Initial pore pressure (kPa):	341.5	Initial effective stress (kPa):	199.5			
Parameters at failure:						
Failure Criterion:	Max. Effective Prir	nciple Stress Ratio				
Axial strain (%):	4.27					
Deviator stress (kPa):	121.3			Principle Str	resses (kPa)	
Excess pore pressure (kPa)): 117.9		σ1	σ1'	σ3	σ3'
Effective principle stress rat	tio: 2.487		320.8	202.9	199.5	81.6





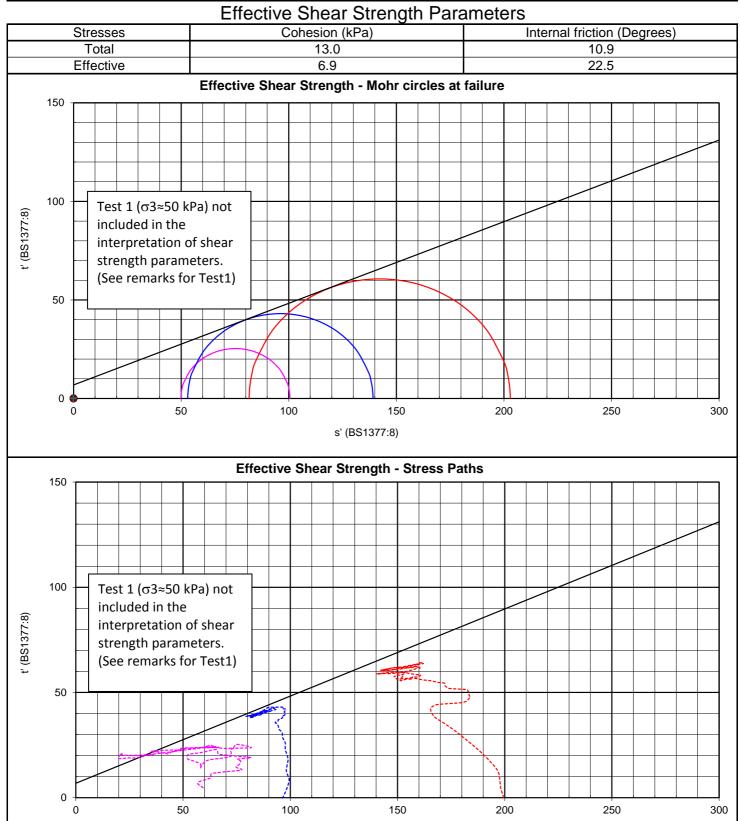
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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-2
Field Sample Reference:	TP 2	Depth (m):	1.8



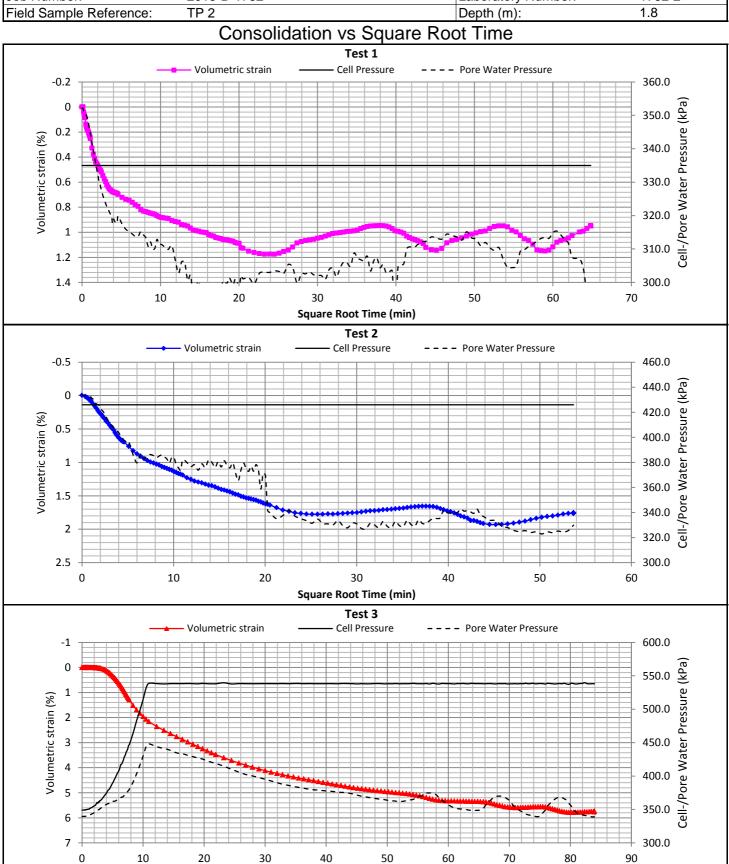
s' (BS1377:8)

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Triaxial Compression Test Results

Project:	Tharisa Minerals	Date Received:	2013/08/02
Job Number:	2013-B-1782	Laboratory Number:	1782-2
Field Sample Reference:	TP 2	Depth (m):	1.8

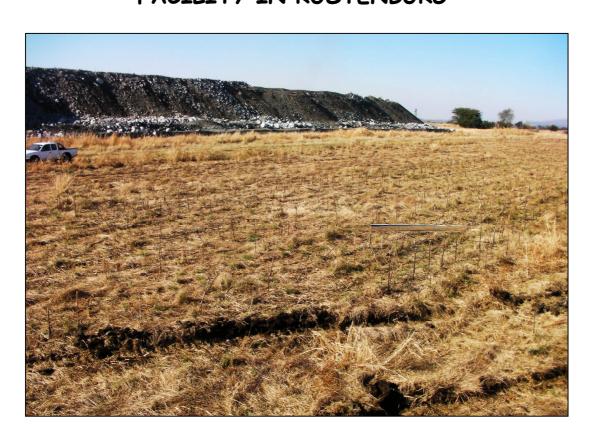


Square Root Time (min)

Reference No: 0996/g



REPORT ON A GEOTECHNICAL INVESTIGATION FOR THE THARISA TAILINGS STORAGE FACILITY IN RUSTENBURG



AUGUST 2011

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REPORT ON A GEOTECHNICAL INVESTIGATION FOR THE THARISA TAILINGS STORAGE FACILITY IN RUSTENBURG

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Geotechnical Investigation	ii	Tharisa Tailing Storage Facility

REPORT ON A GEOTECHNICAL INVESTIGATION FOR THE THARISA TAILINGS STORAGE FACILITY IN RUSTENBURG

1. TERMS OF REFERENCE

This report presents the results of a geotechnical investigation carried out to establish the subsoil and founding conditions within the areas to be occupied by the footprint of two tailings storage facilities (TSF) for the Tharisa mine near Rustenburg. The objectives of the investigation were to:

- establish the thickness and extent of the clay that is known to cover the area;
- determine the shear strength parameters and permeability coefficient of the *in-situ* and remoulded clay;
- provide an estimate of the permeability and shear strength parameters of the highly weathered norite rock or sand underlying the clay horizon.

Inroads Consulting cc was requested by Ms. Georgia Vagis of Epoch Resources (Pty) Ltd to carry out a geotechnical investigation within the two proposed TSF sites.

2. SITE DESCRIPTION

Two sites identified for the tailings storage facilities are located north of the N4 highway near the Buffelspoort Toll Plaza where their ground surfaces fall at fairly shallow gradients in a north and north easterly direction. The ground cover over both sites consists largely of grass and ploughed sunflower fields.

Photographs of the sites terrain are presented in **Appendix A**.

3. INVESTIGATION

The field investigation was carried out between 30th March and 4th April 2011, and again on 13th June 2011. It entailed setting out and excavating 30 test pits employing a Liebherr 974 Excavator.

The position of the holes, referenced TP1 to TP30, are shown on the attached site plan in **Figure1**, and were positioned so as to cover as much of the areas as possible.

The holes were excavated to depths averaging 4,5 m in the range 2,4 to 6,7 m, below which depth the refusal occurred in some of the pits on soft to medium hard rock. They were profiled in accordance with the standard methods and procedures described in the document *Guidelines for Soil and Rock Logging in South Africa*⁽¹⁾.

Soil samples were recovered from certain of the test pit horizons and sent to Civilab, a commercial civil engineering testing laboratory in Johannesburg, for further testing.

The soil profile records for each of the test pits are presented in **Appendix B** of this report.

4. GEOLOGICAL AND SUBSOIL CONDITIONS

According to the 1:50 000 (2527 DA Wolhuterskop) referenced geological map, the area is underlain by norite, quartz norite, quartz gabbro and pyroxenitic norite. Descriptions of the soils encountered are discussed below:

The subsoils encountered are characterised by dark grey or black occasionally dark brown, soft to firm clays which are commonly referred to as "black turf". They are underlain by grey and olive predominantly very soft to medium hard rock gabbro norite and occasional medium dense to dense sand.

These subsoils are described in detail below:

4.1 Western TSF Site

4.1.1 Soil description

Fourteen test pits referenced TP1 to TP4 and TP21 to TP30 were dug within the site and the subsoils encountered are described below:

Residual clay "black turf": blankets the site to an average depth of 1,8 m in the range 1,4 to 3,2 m. It is shattered and slickensided and exhibits a soft consistency. It covers the whole area and is overlain by topsoil comprising soft clay with sand containing numerous roots to a depth of up to 0,2 m below surface.

A 200 mm thick layer of soft to firm, sandy clay occurs sporadically within the site and represents the transition between the "black turf" and the underlying soft rock or residual sand. This frequently comprise minor to abundant fine and medium calcrete and gabbro norite gravels.

The shattered and slickensided structure suggests that the "black turf" is potentially highly active.

Gabbro norite rock: underlies the "black turf" and is predominantly grey and olive, stained black and brown, closely jointed and breaks into angular gravels and sand. In places, however, the rock has weathered to a medium dense to dense sand with minor to abundant angular gravels exhibiting the relict structure of the rock. With depth the strength or consistency of the rock, which is predominantly very soft to soft, improves and the backacter refused within it.

The soil profile records for each of the test pits are presented in **Appendix B**.

4.1.2 Groundwater

Groundwater seepage was encountered in test pits TP21 and TP30 at a depth of 4,2 and 3,6 m respectively, with water standing at these depths after 24 hours.

4.2 Eastern TSF Site

4.2.1 Soil description

Sixteen test pits referenced TP5 to TP20, were excavated within the site and the subsoils encountered include the following:

Residual clay "Black turf": is predominantly shattered and slickensided and covers the site to an average depth of 4,2 m in the range 0,7 m to 6,2 m. This comprises soft to firm dark grey or black

clay which becomes firm and dark brown with depth and contains minor to abundant fine and medium calcrete gravels.

Residual norite sand: occurs sporadically within the site in test pits TP5, TP6, TP10, TP11, TP12 and TP18 and occurs below the "black turf" where it comprises dense to very dense sand which is of a very dense to very soft rock consistency in places.

Norite rock: underlies the "black turf" and the residual sand described above, and is predominantly grey and olive speckled white, stained orange and brown, closely jointed and breaks into angular gravels and sand. With depth the strength or consistency of the rock or residual soils improves and the backacter partially refused within it.

The soil profile records for each of the test pits are presented in **Appendix B** of this report.

4.2.2 Groundwater

No groundwater seepage was encountered within any of the pits.

5. LABORATORY TEST RESULTS

The results of the laboratory tests carried out on selected soil samples recovered from the test pits are summarised below and presented in **Appendix C**.

5.1 Foundation Indicator

For more accurate identification and for classification purposes, particle size distribution analysis and Atterberg limit determinations were carried out on samples of the near surface residual clay and sand and rock fragments and the results of these are summarized in **Table 1**.

The residual gabbro norite clay "black turf" classifies as "CH" soil type according to the Unified Soil Classification (USC) system, this being clay of high plasticity.

According to the AASHTO classification system the residual sand and very soft rock gabbro norite classifies as an A-1-b soil type this being sand with stone fragments.

The plasticity index (PI) for the residual clay averages 52 ranging from 32 to 72, and when weighted averages 50. This suggests that it is of high to very high expansive potential according to the method of Van der Merwe ⁽²⁾. With an average grading modulus of 0,16, the soil is fine grained and comprises largely of the clay fraction.

The residual sand and rock fragments is non plastic (NP) and slightly plastic (SP) with a grading modulus of 1,54 to 2,25 respectively which is coarse.

5.2 Compaction Tests

Standard Proctor compaction tests were carried out on representative samples of the residual sand and gabbro norite rock fragments within the TSF sites, mainly to determine their compaction characteristics in terms of the maximum dry density and optimum moisture content for remoulding in the triaxial apparatus for strength and permeability estimates.

The results of these tests are presented in **Table 2** overleaf.

Samples of the sand and rock fragments tested are all fairly uniform with optimum moisture contents of 12,4% to 14,3% for the sand and 8,2% for the rock fragments respectively, which is typical of a relatively finer grained sand and much coarser rock fragments sampled. The maximum Proctor dry densities for the sand are between 1940 kg/m³ and 2271 kg/m³ and for the rock fragments is 2197 kg/m³.

Table 1: Summary of results of indicator tests

TSF Site	Test	Description	Depth	LL	PI	Pl _{ws}	LS	GM	МІТ	MIT Size Fraction - %			Class	sification
13F 3ite	pit No.	Description	(m)	LL	FI	Flws	L	GIVI	Gravel	Sand	Silt	Clay	USC	AASHTO
Western TSF Site	TP2	Residual gabbro norite - Sand	1.6 – 4.0	NP	NP	NP	0.0	1.54	7	85	6	2	SP-SM	A-1-b (0)
Western TSF Site	TP4	Residual gabbro norite - Clay	1.0 – 1.2	63	34	33	17.5	0.15	1	15	28	57	СН	A-7-6 (20)
Eastern TSF Site	TP6	Residual gabbro norite - Clay	1.0 – 1.2	60	32	31	15.0	0.06	0	7	25	68	СН	A-7-6 (20)
Eastern TSF Site	TP6	Very soft rock gabbro norite	4.7 – 5.1	SP	SP	SP	1.0	2.25	48	48	3	1	SW-SM	A-1-b (0)
Eastern TSF Site	TP9	Residual gabbro norite - Clay	4.5 – 4.7	105	72	72	22.0	0.04	0	8	28	64	СН	A-7-5 (20)
Eastern TSF Site	TP10	Residual gabbro norite - Clay	4.0 – 4.2	98	64	62	20.0	0.13	1	13	22	64	СН	A-7-5 (20)
Eastern TSF Site	TP14	Residual gabbro norite - Clay	4.0 – 4.2	93	58	50	20.5	0.43	12	9	27	52	СН	A-7-5 (20)
Eastern TSF Site	TP19	Residual gabbro norite - Sand	1.2 – 3.4	NP	NP	NP	0.0	2.25	44	53	2	1	SW	A-1-b (0)

LL = liquid limit; PI = plasticity index; PI_{ws} = plasticity index of whole sample; LS = linear shrinkage; GM = grading modulus, USC = unified soil classification, AASHTO = American Association of State Highway and Transportation Officials; MIT = Massachusetts Institute of Technology

Table 2: Summary of Results of Compaction tests – Proctor

Hole No.	Description	Depth (m)	ρ _d max (kg/m³)	omc (%)
TP2	Residual gabbro norite - Sand	1.6 – 4.0	1940	14.3
TP6	Very soft rock gabbro norite	4.7 – 5.1	2197	8.2
TP19	Residual gabbro norite - Sand	1.2 – 3.4	2271	12.4

 ρ_{d} max = maximum dry density; omc = optimum moisture content;

5.3 Triaxial test

Undisturbed samples of the residual clay "black turf", and remoulded sand and rock fragments samples were subjected to consolidated undrained and drained triaxial tests respectively, and the results are summarized in **Table 3** below. Both the drained and undrained tests were carried out at cell pressures of approximately 50, 100 and 200 kPa and pore water pressure measurements were taken during shearing for the consolidated undrained tests.

The residual clay tested has in excess of 52 % clay by mass and in terms of effective stresses exhibits cohesion of 20 to 30 kPa and a friction angle of 12 degrees. The remoulded sand and rock fragments exhibit an apparent cohesion of 3 kPa and an angle of friction of between 35 to 43 degrees.

5.4 Permeability Tests

Permeability tests were carried out on samples of the residual clay "black turf" and the remoulded residual sand and rock fragments prepared in the triaxial cell, and the results are summarized in **Table 4** below.

The permeability coefficients for the remoulded sand and rock fragments are in the range 3,0 x 10^{-7} and 2,7 x 10^{-6} m/sec, which classify these soil as being slightly permeable when remoulded to 95% proctor compactive effort. On the other hand the "black turf" is regarded as almost impermeable with the permeability coefficients of between 4.7 x 10^{-10} and 1.0 x 10^{-9} .

Table 3: Summary of triaxial tests

Hole No.	Depth (m)	Soil Description	Remoulded/ Undisturbed	Type of Test	Dry density (kg/m³)	Moisture content (%)	Cohesion (kPa) c'	Angle of friction (degrees) φ'
TP2	1.6 – 4.0	Residual gabbro norite - Sand	Remoulded	CD	1820	14.6	22	35
TP4	1.0 – 1.2	Residual gabbro norite - Clay	Undisturbed	CU	1259	33.7	20	12
TP6	4.7 – 5.1	Very soft rock gabbro norite	Remoulded	CD	2051	9.5	15	38
TP10	4.0 – 4.2	Residual gabbro norite - Clay	Undisturbed	CU	1299	38.4	30	11
TP19	1.2 – 3.4	Residual gabbro norite - Sand	Remoulded	CU	1994	14.0	3	43

CD = Consolidated Drained; CU = Consolidated Undrained with measurements of pore water pressures

Table 4: Summary of permeability tests

Hole No.	Depth (m)	Soil Description	Dry density (kg/m³)	Moisture Content (%)	Coefficient of permeability (m/sec)
TP2	1.6 – 4.0	Residual gabbro norite - Sand	1830	14.1	4.5 x 10 ⁻⁶
TP4	1.0 – 1.2	Residual gabbro norite - Clay	1259	33.7	9.1 x 10 ⁻¹⁰
TP6	1.0 – 1.2	Residual gabbro norite - Clay	1298	36.4	4.7 x 10 ⁻¹⁰
TP6	4.7 – 5.1	Very soft rock gabbro norite	1933	10.6	3.0 x 10 ⁻⁷
TP9	4.5 – 4.7	Residual gabbro norite - Clay	1222	38.2	1.0 x 10 ⁻⁹
TP14	4.0 – 4.2	Residual gabbro norite - Clay	1121	45.2	5.4 x 10 ⁻¹⁰
TP19	1.2 – 3.4	Residual gabbro norite - Sand	2209	16.9	2.7 x 10 ⁻⁶

6. GEOTECHNICAL ASSESSMENT AND RECOMMENDATIONS

Based on the detailed profiles of 30 tests pits excavated within the area identified for the design and construction of the TSF's, together with the results of laboratory tests, the following findings and recommendation are provided.

6.1.1 Subsoil Conditions

Both the Western and Eastern TSF sites are covered by the residual clay "black turf" to depths in the range 1,4 to 3,2 m and 0,7 to 6,2 m respectively. Laboratory tests indicate that the clayey horizons are of very low permeability, typically in the order 10^{-9} metres per second and, depending on their thickness, should contribute significantly to reducing seepage into the underlying sand and rock strata. The underlying rock sand rock strata appear to be relatively permeable from the soil profiles, however, the remoulded samples tested indicates that, based on the coefficient of permeability in the range $3,0 \times 10^{-7}$ and $2,7 \times 10^{-6}$ m/sec, they are "slightly permeable"

6.1.2 Impoundment Wall

Rock Fill

It is understood that the TSF's will be constructed of rock fill to form a perimeter impoundment wall. This configuriguration will require substantial fill with which to construct the impoundment. It is estimated that about 1.0 Mm³ and 3.5 Mm³ of residual clay can be sourced from within the basin of the Western and Eastern TSF's respectively, with which a clay lining can be formed.

Filter

By virtue of its coarse grained composition and concomitant relatively large voids, the rock fill must be considered pervious, in which case the upstream face should be provided with a suitable filter blanket to prevent tailings piping into and through the rock fill wall. Consideration should be given to placing a protective layer of earth over the filter to protect it from damage through scour erosion during tailings deposition. If possible, during operations coarse tailings should be carefully deposited against the filter under controlled conditions.

Stability

Stability of the wall by shear failure through the underlying clay and sliding along the dense residual gabbro norite underlying foundation and the tailings should follow conventional methods, including rotational and sliding failure.

For analysis purposes the clayey layer should be assumed to extend to a maximum depth of 6,0 m below surface and shear strength parameters of c' = 10 kPa and $\phi' = 12^{\circ}$ should be used for analysis. Design shear strength parameters for the sand and rock fragments of c' = 0 kPa and $\phi' = 33^{\circ}$ are considered reasonable.

7. REFERENCES

- Geoterminology Workshop (1990), Guidelines for Soil and Rock Logging in South Africa.
 Association of Engineering Geologists, Geotechnical Division of the South African Institute of Civil Engineering and South African Institute of Engineering Geologists.
- 2) Van der Merwe, D.H. *The Prediction of Heave from the Plasticity Index and Percentage Clay Fraction.* Trans. S.A. Ins. Civ. Eng. No. 6, 1964.

Moruti Shuping B-Tech (Geotech)

for Inroads Consulting cc

Brian Harrison Pr Eng

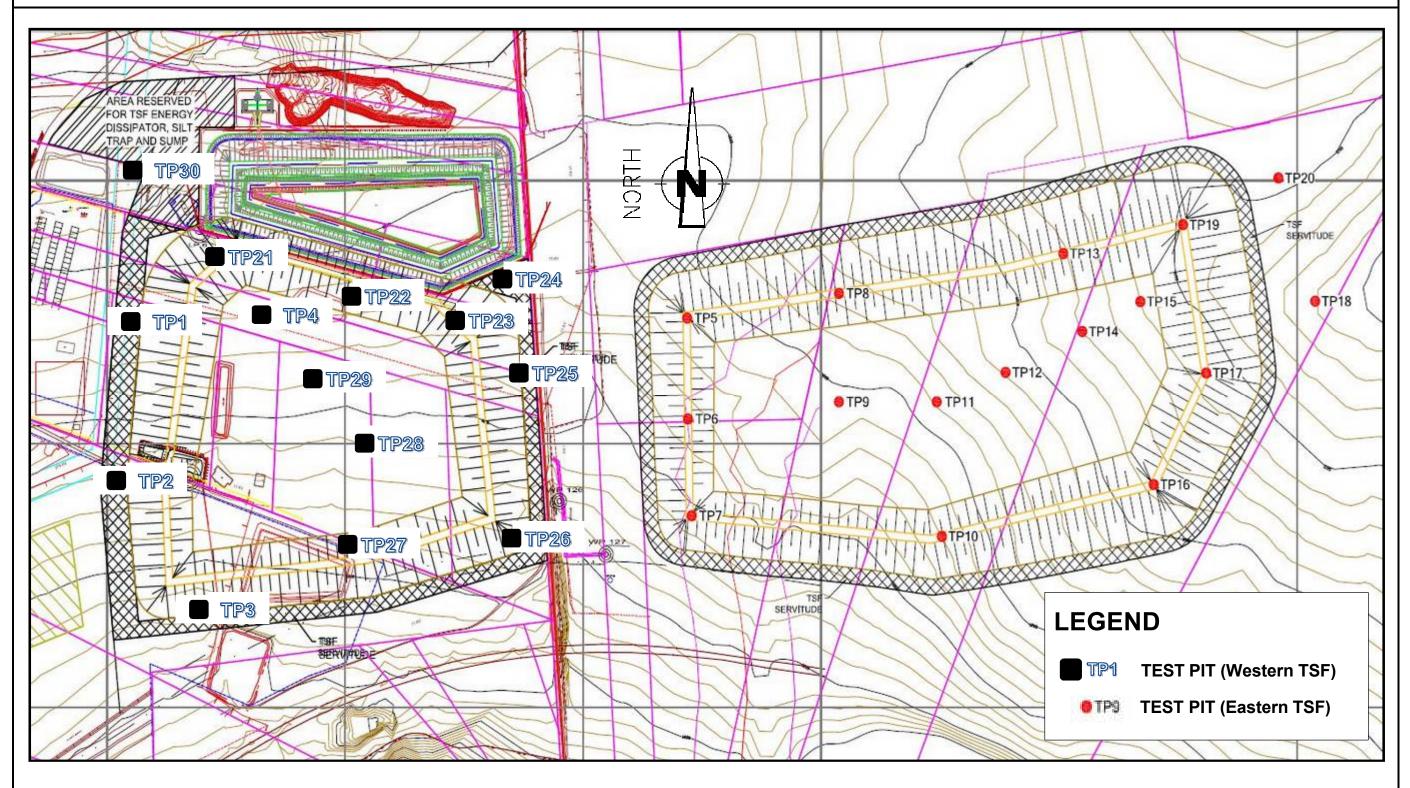
for Inroads Consulting cc



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EPOCH RESOURCES (PTY) LTD.
THARISA MINERALS TAILINGS STORAGE FACILITIES
SITE PLAN SHOWING POSITIONS OF TEST PITS

Figure No 1

Inroads Consulting		Ref: 0996/
	APPENDIX A	
PHO	TOGRAPHS OF	SITE



View of the Western TSF site from near TP4 looking south west.



View of the Eastern TSF site from the southern border looking north.

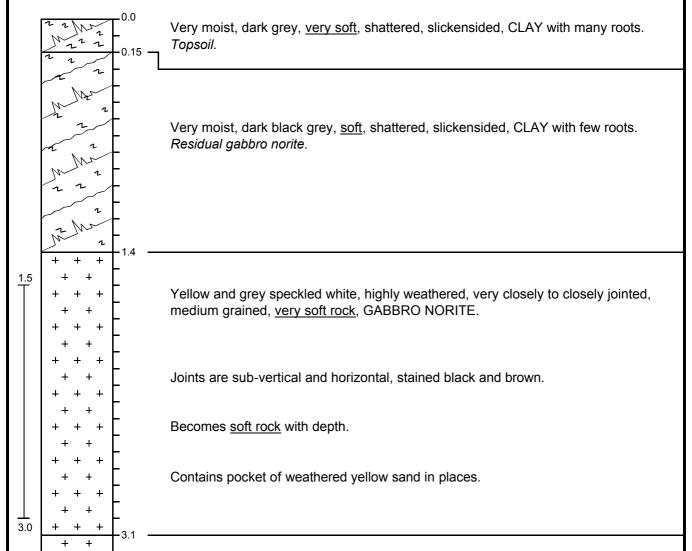
Inroads Consulting		Ref: 0996/g
	APPENDIX B	
	AFFERDIA D	
	SOIL PROFILES	



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PROFILE SHEET TP1 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -050602



NOTES:

- 1. Bottom of hole at 3,1 m. Partial refusal on soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Bulk sample taken from 1,5 to 3,0 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.



Water seepage

Standing water

Undisturbed sample

Disturbed sample

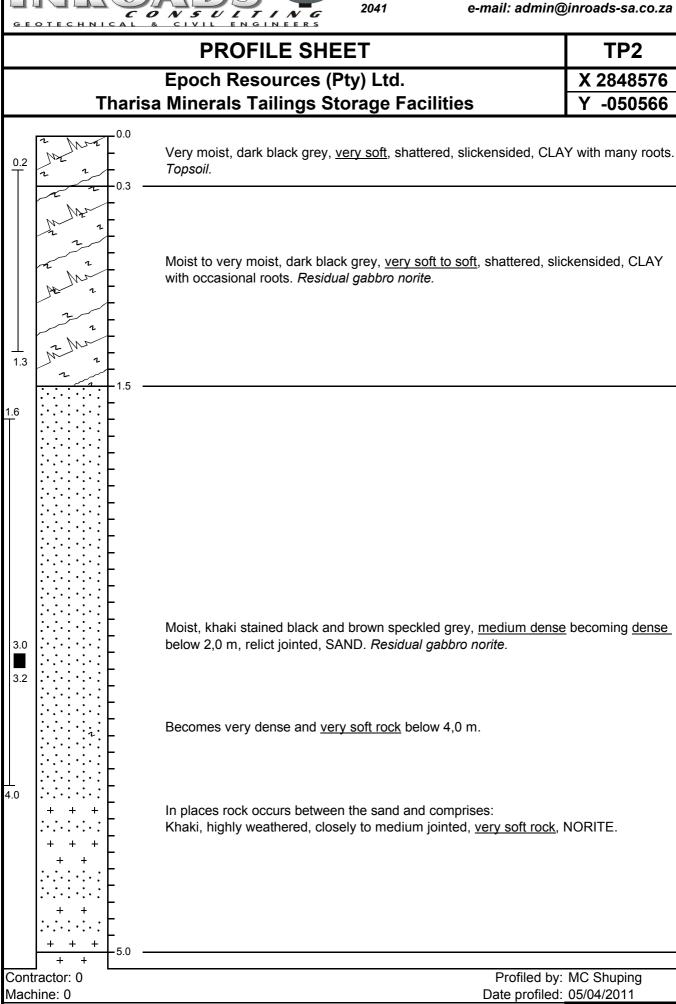
Bulk sample

In-situ test

Ref: 0996/g

Sheet 1 of 2

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PROFILE SHEET	TP2 cont
Epoch Resources (Pty) Ltd.	X 2848576
Tharisa Minerals Tailings Storage Facilities	Y -050566



NOTES:

- 1. Bottom of hole at 5,3 m. Partial refusal on <u>very dense to very soft rock</u> GABBRO NORITE.
- 2. Very slow water seepage encountered below 2,0 m. Water standing at the bottom of the pit after overnight.
- 3. Undisturbed sample taken from 3,0 to 3,2 m.
- 4. Bulk sample taken from 0,2 to 1,3 m and 1,6 to 4,0 m.
- 5. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 05/04/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample

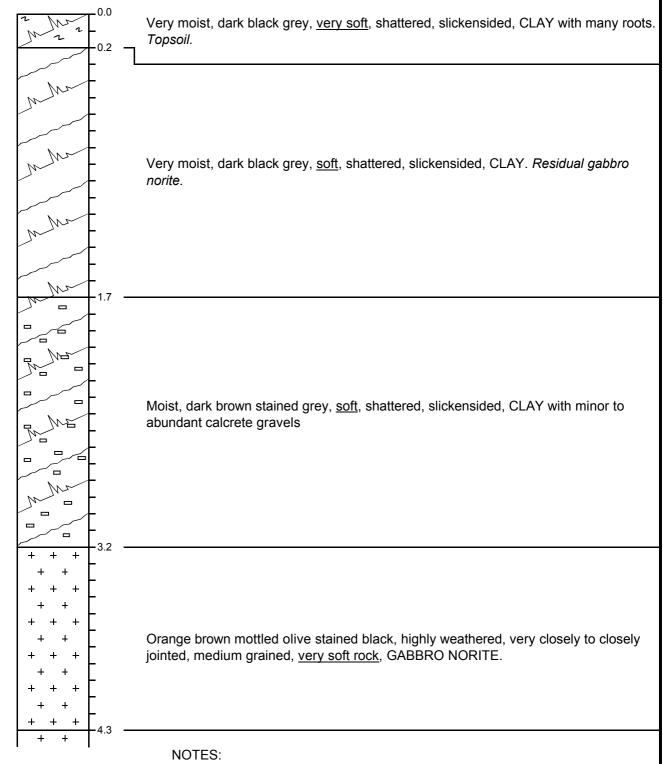
Ref: 0996/g



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PROFILE SHEET TP3 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -050759



- 1. Bottom of hole at 4,3 m. Partial refusal on soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0

Machine: 0

Water seepage

✓ Standing water

✓ Disturbed sample

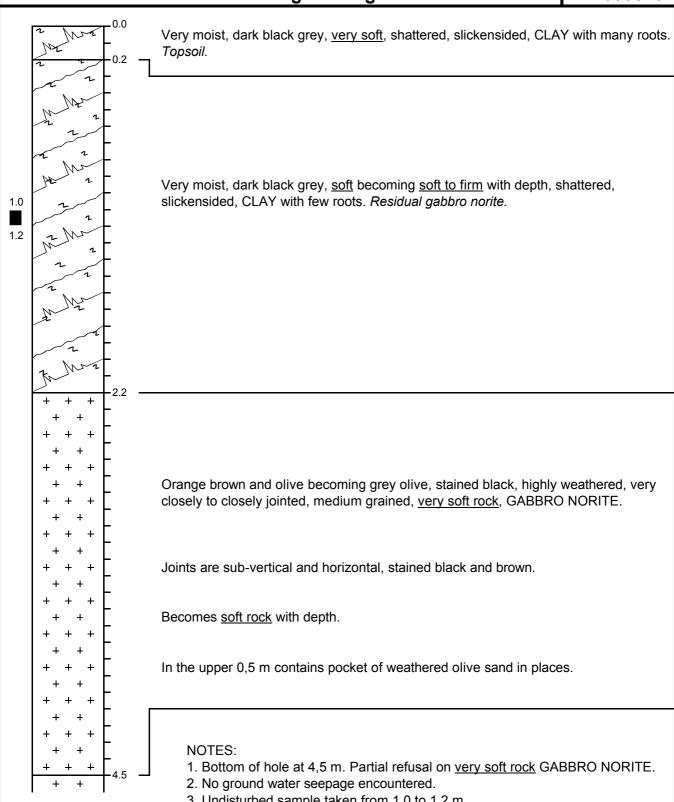
✓ Standing water



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PROFILE SHEET TP4 **Epoch Resources (Pty) Ltd.** X 2848264 **Tharisa Minerals Tailings Storage Facilities** -050828



- 3. Undisturbed sample taken from 1,0 to 1,2 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

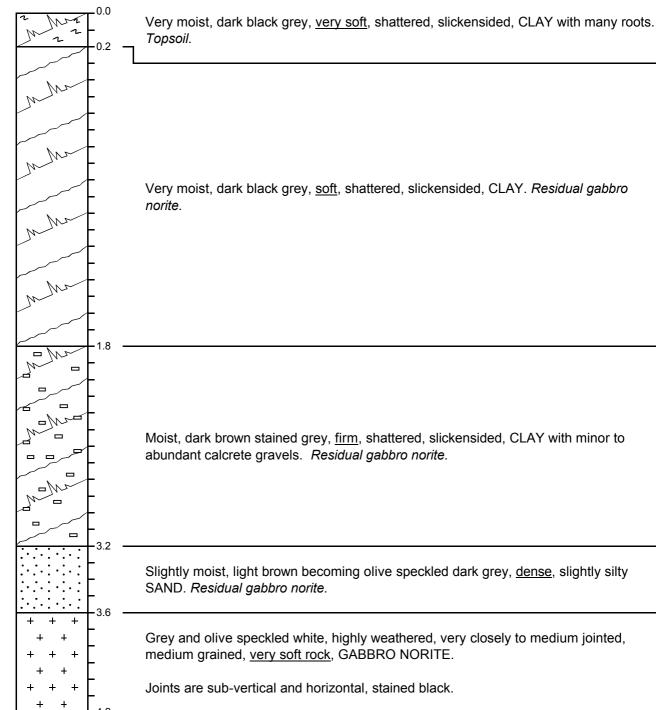
Contractor: 0 Profiled by: MC Shuping Date profiled: 05/04/2011 Machine: 0 Water seepage Undisturbed sample Bulk sample Ref: 0996/g Standing water Disturbed sample In-situ test Sheet 1 of 1



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PROFILE SHEET	TP5
Epoch Resources (Pty) Ltd.	X 2848265
Tharisa Minerals Tailings Storage Facilities	Y -051715



Slightly moist, light brown becoming olive speckled dark grey, dense, slightly silty

Grey and olive speckled white, highly weathered, very closely to medium jointed,

NOTES:

- 1. Bottom of hole at 4,2 m. Partial refusal on soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping Date profiled: 30/03/2011 Machine: 0 Water seepage Ref: 0996/g Undisturbed sample Bulk sample Standing water Disturbed sample In-situ test Sheet 1 of 1

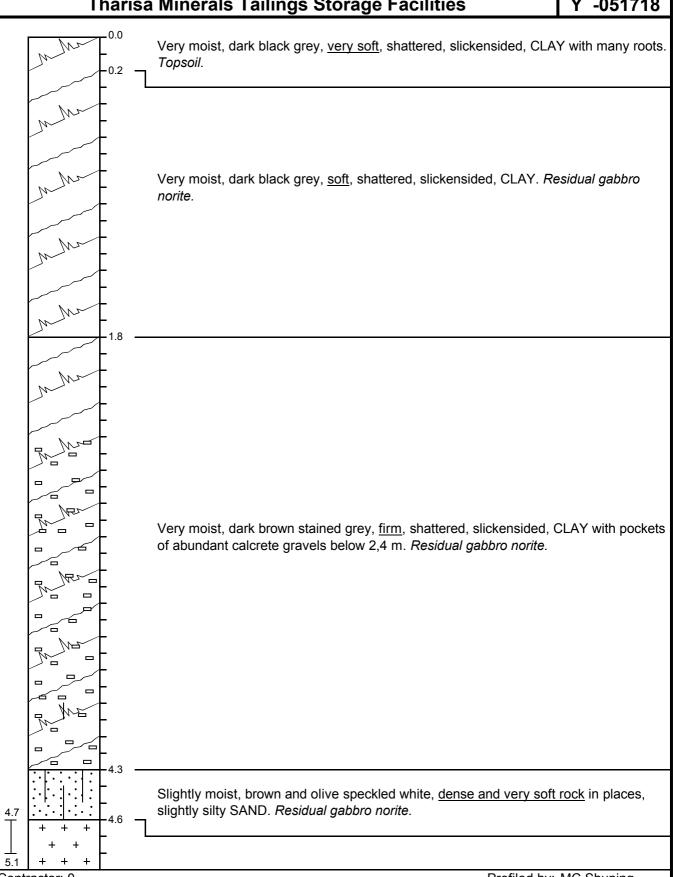


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PROFILE SHEET
TP6

Epoch Resources (Pty) Ltd.
Tharisa Minerals Tailings Storage Facilities
Y -051718



Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 30/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

☐ Bulk sample ☐ In-situ test

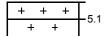
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PROFILE SHEET	TP6 cont
Epoch Resources (Pty) Ltd.	X 2848451
Tharisa Minerals Tailings Storage Facilities	Y -051718



Grey and olive speckled white, highly weathered, very closely to medium jointed, medium grained, <u>very soft rock</u>, GABBRO NORITE.

Joints are sub-vertical and horizontal, stained black.

NOTES:

- 1. Bottom of hole at 5,1 m. Partial refusal on very soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Bulk sample taken from 4,7 to 5,1 m.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 30/03/2011

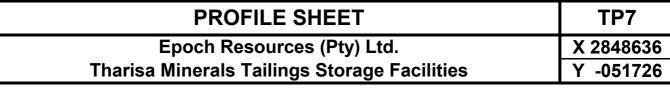
✓ Water seepage Undisturbed sample ☐ Bulk sample

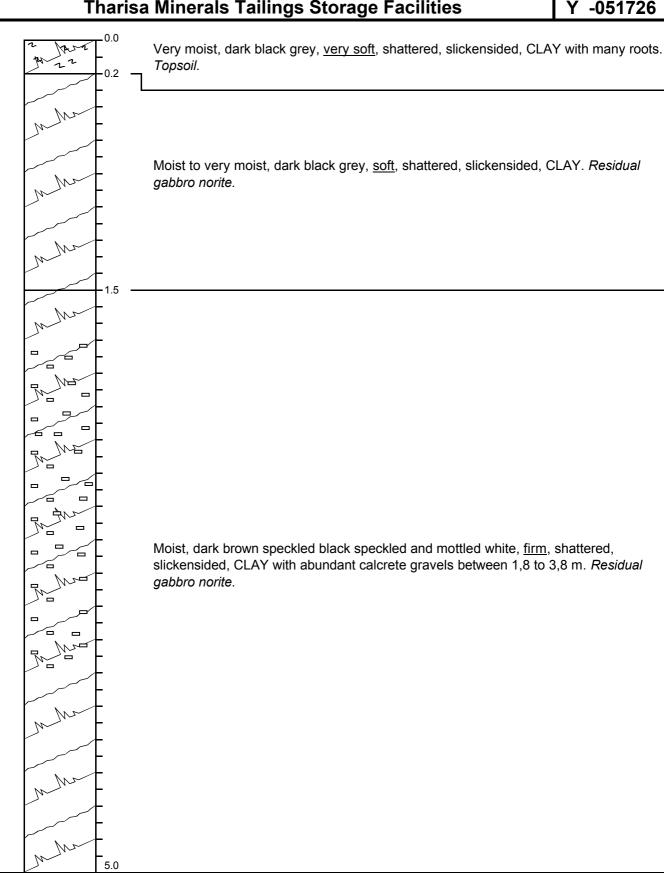
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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 30/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

☐ Bulk sample ☐ In-situ test

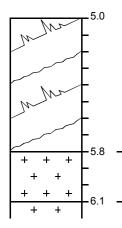
Ref: 0996/g Sheet 1 of 2



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PROFILE SHEET	TP7 cont
Epoch Resources (Pty) Ltd.	X 2848636
Tharisa Minerals Tailings Storage Facilities	Y -051726



Standing water

Moist, dark brown speckled black speckled and mottled white, <u>firm</u>, shattered, slickensided, CLAY. *Residual gabbro norite*.

Olive mottled orange, speckled white, highly weathered, very closely, medium grained, very soft rock, GABBRO NORITE.

Joints are sub-vertical and horizontal, stained black and brown.

NOTES:

- 1. Bottom of hole at 6,1 m on very soft rock GABBRO NORITE. Not to refusal.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

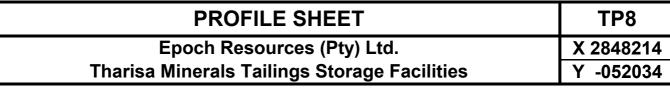
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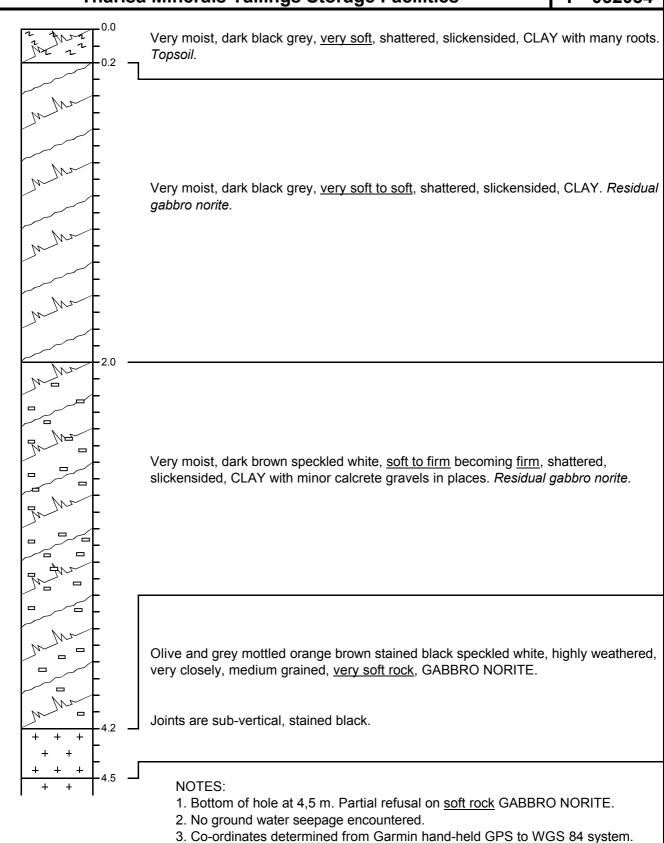
✓ Water seepage Undisturbed sample ☐ Bulk sample Ref: 099



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Disturbed sample

Standing water

Bulk sample

In-situ test

Date profiled: 31/03/2011

Ref: 0996/g

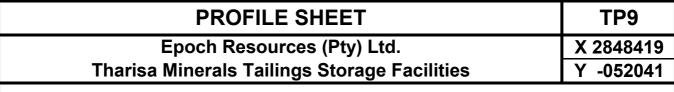
Sheet 1 of 1

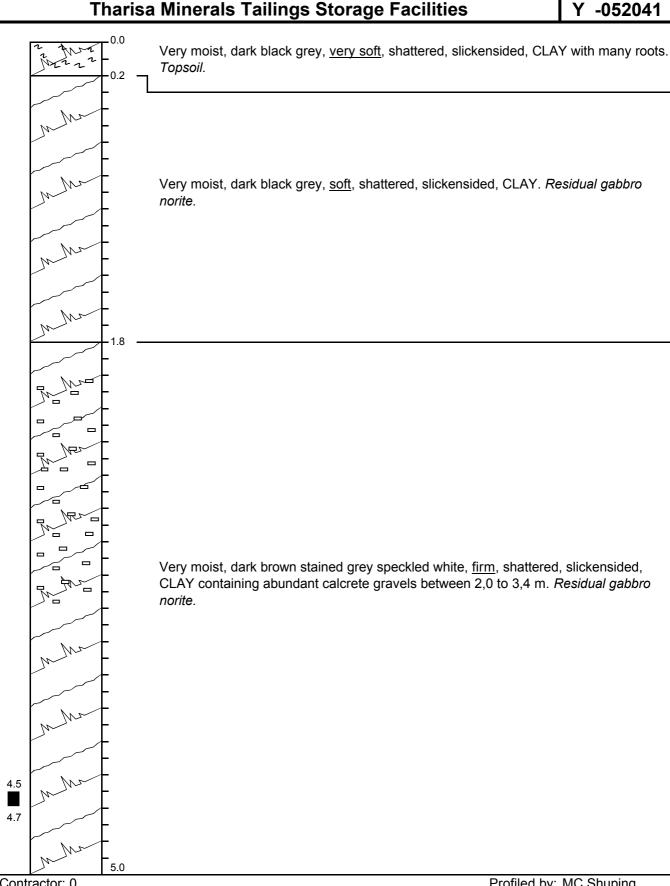
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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 04/04/2011

✓ Water seepage✓ Standing water

Undisturbed sampleDisturbed sample

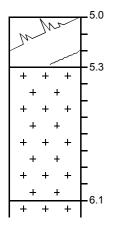
☐ Bulk sample In-situ test



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PROFILE SHEET	TP9 cont
Epoch Resources (Pty) Ltd.	X 2848419
Tharisa Minerals Tailings Storage Facilities	Y -052041



Very moist, dark brown stained grey speckled white, <u>firm</u>, shattered, slickensided, CLAY. *Residual gabbro norite*.

Olive becoming grey, highly weathered, very closely to medium jointed, medium grained, <u>very soft rock</u>, GABBRO NORITE.

Joints are sub-vertical and horizontal, stained black.

NOTES:

- 1. Bottom of hole at 6,1 m. Partial refusal on <u>very soft to soft rock</u> GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Undisturbed sample taken from 4,5 to 4,7 m.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 04/04/2011

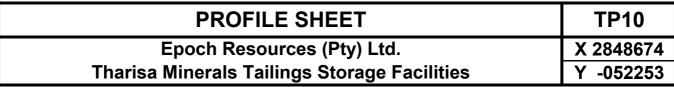
✓ Water seepage Undisturbed sample ☐ Bulk sample

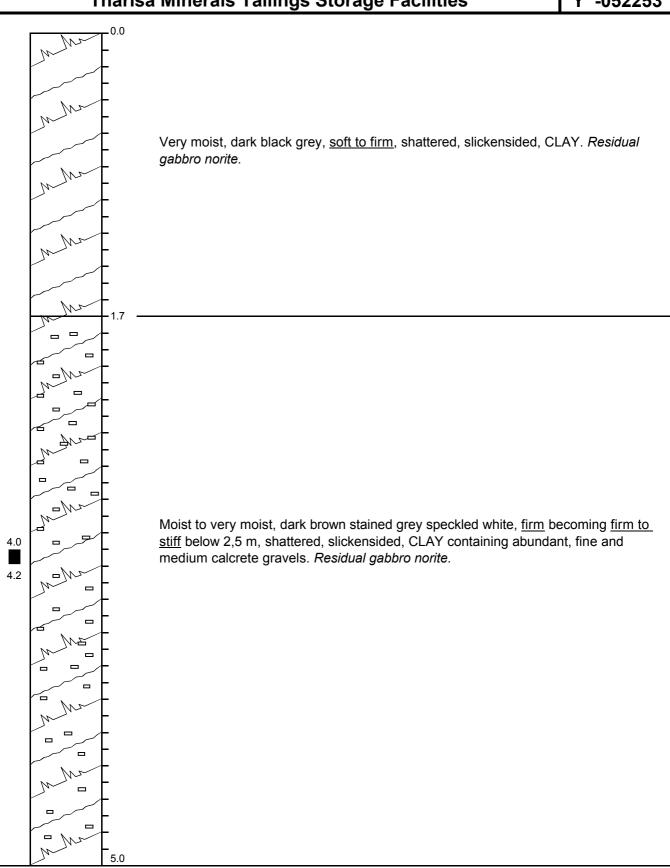
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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 31/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

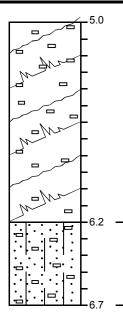
☐ Bulk sample In-situ test



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PROFILE SHEET	TP10 cont
Epoch Resources (Pty) Ltd.	X 2848674
Tharisa Minerals Tailings Storage Facilities	Y -052253



Moist to very moist, dark brown stained grey speckled white, <u>firm</u> becoming <u>firm to stiff</u> below 2,5 m, shattered, slickensided, CLAY containing abundant, fine and medium calcrete gravels. *Residual gabbro norite*.

Slightly moist to moist, brown speckled white, <u>medium dense to dense</u>, slightly clayey SILTY SAND. *Residual gabbro norite*.

Contains pockets of 50 mm thick clay and abundant calcrete gravels.

NOTES:

- 1. Bottom of hole at 6,7 m on <u>medium dense to dense</u> silty sand and abundant calcrete gravels. Not to refusal.
- 2. No ground water seepage encountered.
- 3. Undisturbed sample taken from 4,0 to 4,2 m.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 31/03/2011

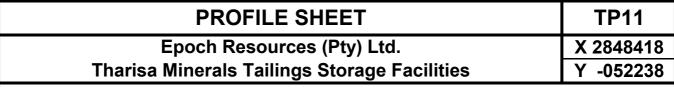
✓ Water seepage Undisturbed sample ☐ Bulk sample

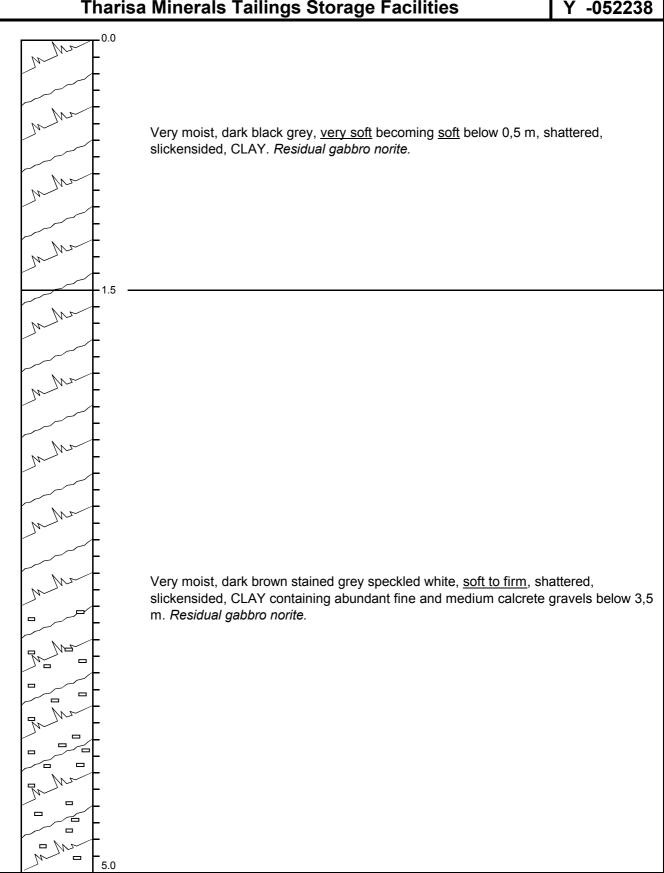
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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 31/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

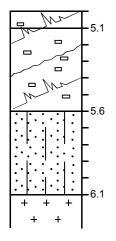
☐ Bulk sample ☐ In-situ test



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PROFILE SHEET	TP11 cont
Epoch Resources (Pty) Ltd.	X 2848418
Tharisa Minerals Tailings Storage Facilities	Y -052238



Moist, grey brown stained grey, <u>firm</u>, shattered, slickensided, CLAY with abundant fine, medium and coarse calcrete gravels. *Residual gabbro norite*.

Moist, olive speckled white, mottled orange brown stained black, <u>very dense</u> silty sand. *Residual gabbro norite*.

In places rock occurs between the sand and comprises:
Olive, highly weathered, medium and closely jointed, very soft rock, NORITE.

NOTES:

- 1. Bottom of hole at 6,1 m on <u>very dense to very soft rock</u> residual GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 31/03/2011

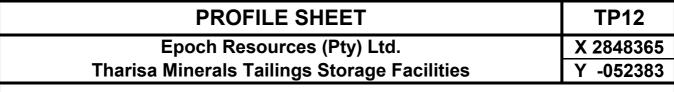
✓ Water seepage Undisturbed sample ☐ Bulk sample

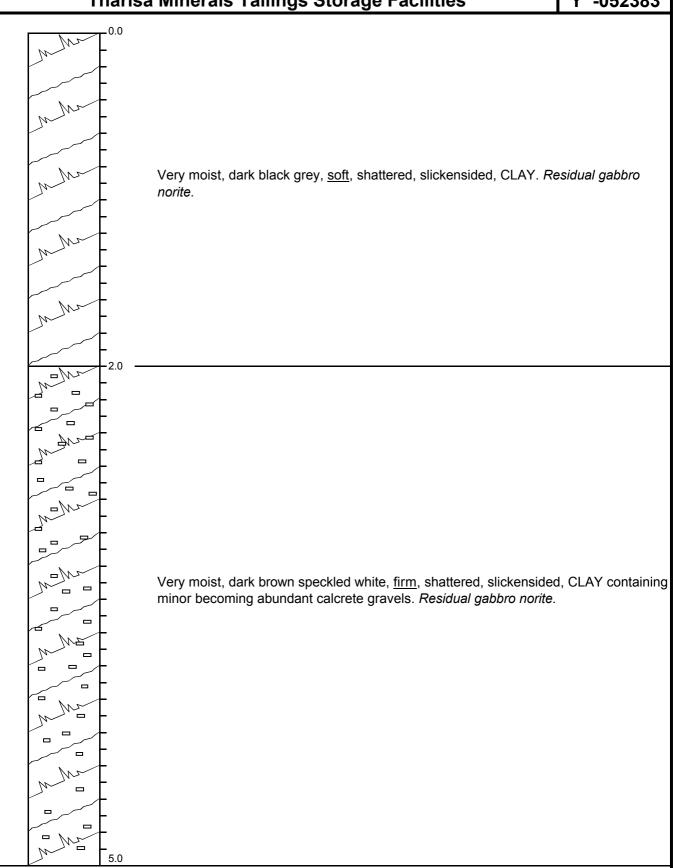
Ref: 0996/g



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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 31/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

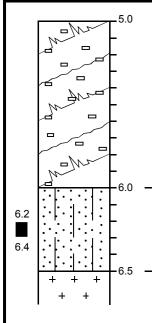
☐ Bulk sample☐ In-situ test



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PROFILE SHEET	TP12 cont
Epoch Resources (Pty) Ltd.	X 2848365
Tharisa Minerals Tailings Storage Facilities	Y -052383



Very moist, dark brown speckled white, <u>firm</u>, shattered, slickensided, CLAY containing minor becoming abundant calcrete gravels. *Residual gabbro norite*.

Moist, olive and grey speckled white stained black, <u>very dense</u>, clayey SILTY SAND. *Residual gabbro norite.*

In places rock occurs abundantly with depth and between the sand and comprises:

Olive and grey, highly weathered, medium and closely jointed, <u>very soft rock</u>, NORITE.

NOTES:

- 1. Bottom of hole at 6,5 m on <u>very dense to very soft rock</u> residual GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Undisturbed sample taken from 6,2 to 6,4 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 31/03/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample

Ref: 0996/g

In-situ test

Sheet 2 of 2

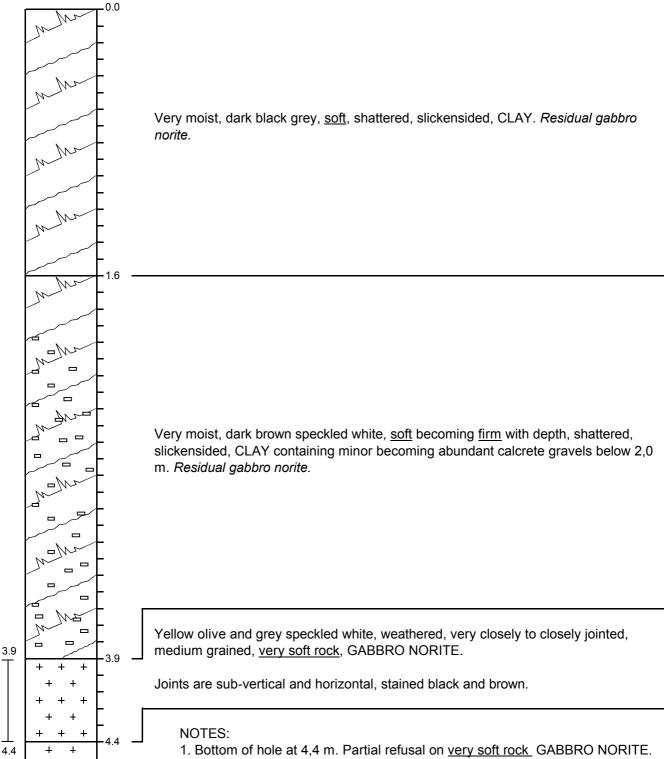
Disturbed sample



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PROFILE SHEET	TP13
Epoch Resources (Pty) Ltd.	X 2848138
Tharisa Minerals Tailings Storage Facilities	Y -052506

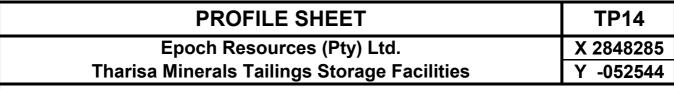


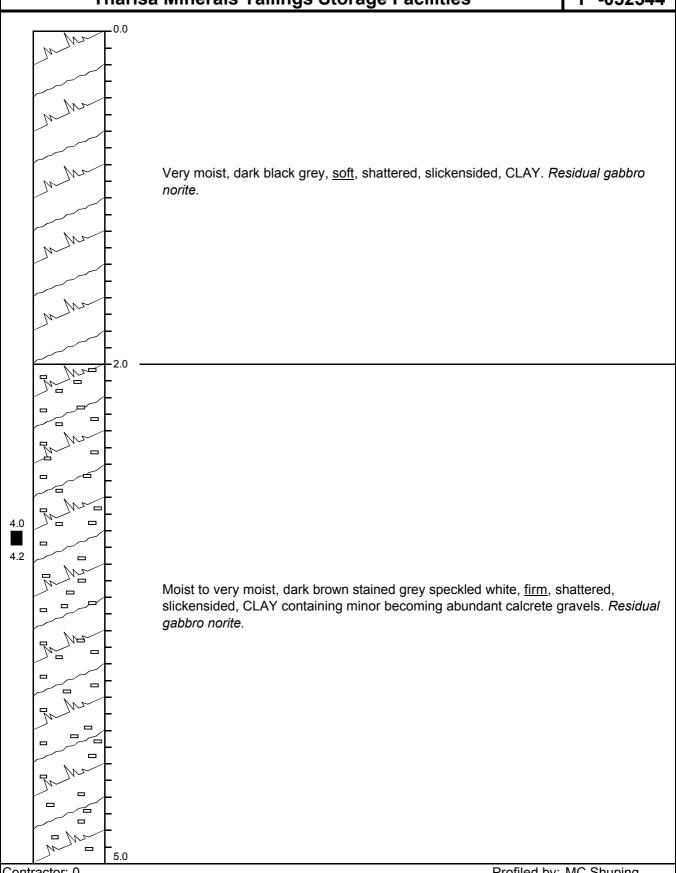
- 2. No ground water seepage encountered.
- 3. Bulk sample taken from 3,9 to 4,4 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.



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Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 31/03/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

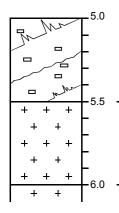
☐ Bulk sample
☐ In-situ test



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PROFILE SHEET	TP14 cont
Epoch Resources (Pty) Ltd.	X 2848285
Tharisa Minerals Tailings Storage Facilities	Y -052544



Closely packed, angular and sub-angular, fine, medium and coarse, calcrete GRAVELS and powder in a matrix of moist, dark grey, clay. *Calcrete/Residual gabbro norite*.

Overall consistency is dense.

White and grey stained black and yellow orange, weathered, very closely to medium jointed, medium grained, very soft to soft rock, GABBRO NORITE.

Joints are sub-vertical and horizontal, stained black.

NOTES:

- 1. Bottom of hole at 6,0 m. Partial refusal on soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping Date profiled: 31/03/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample Ref: 099

Water seepageStanding water

Disturbed sample

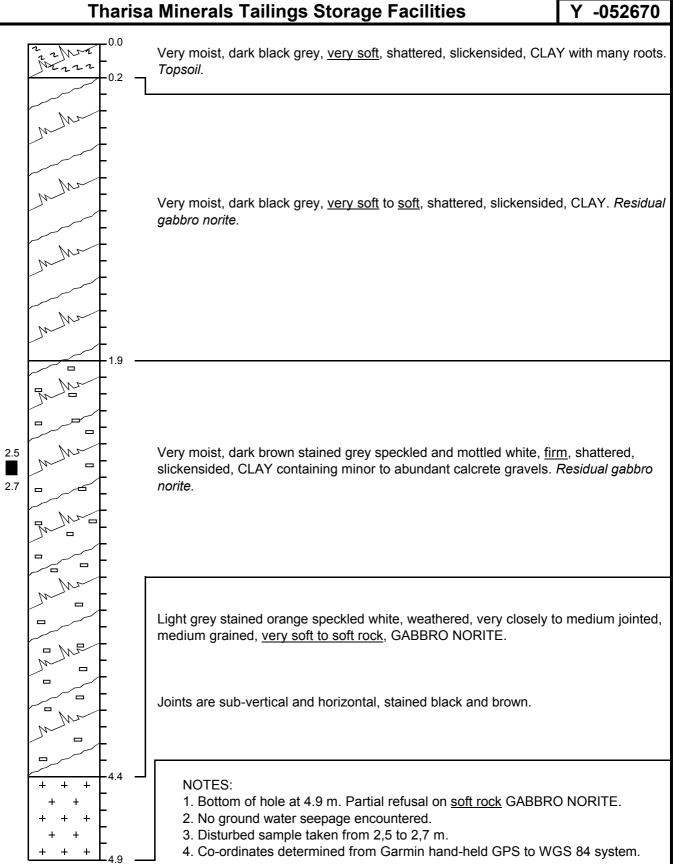
☐ Bulk sample☐ In-situ test



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PROFILE SHEET TP15 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -052670

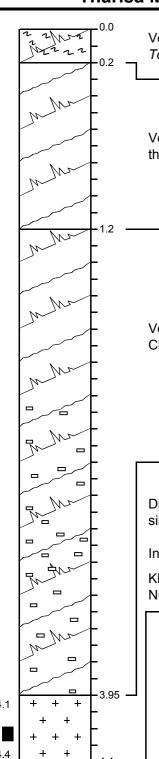




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PROFILE SHEET TP16 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -052696



Very moist, dark black grey, <u>very soft</u>, shattered, slickensided, CLAY with many roots. *Topsoil*.

Very moist, dark black grey, <u>soft</u>, shattered, slickensided, CLAY with many roots on the upper 0,3 m. *Residual gabbro norite*.

Very moist, dark brown stained grey speckled white, <u>firm</u>, shattered, slickensided, CLAY with abundant calcrete gravels below 2,3 m. *Residual gabbro norite*.

Dry to slightly moist, khaki speckled grey and black, <u>medium dense to dense</u>, slightly silty SAND.

In places rock occurs abundantly with depth and between the sand and comprises:

Khaki speckled grey, highly weathered, medium and closely jointed, <u>very soft rock</u>, NORITE.

NOTES:

- Bottom of hole at 4,4 m. Partial refusal on <u>very soft to soft rock</u> GABBRO NORITE.
- 2. Undisturbed sample taken from 4,1 to 4,3 m.
- 3. Bulk sample taken from 4,1 to 4,4 m.
- 4. No ground water seepage encountered.
- 5. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 04/04/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample

Ref: 0996/g

Standing water

Disturbed sample

__ Bulk sampl ---□ *In-situ* test

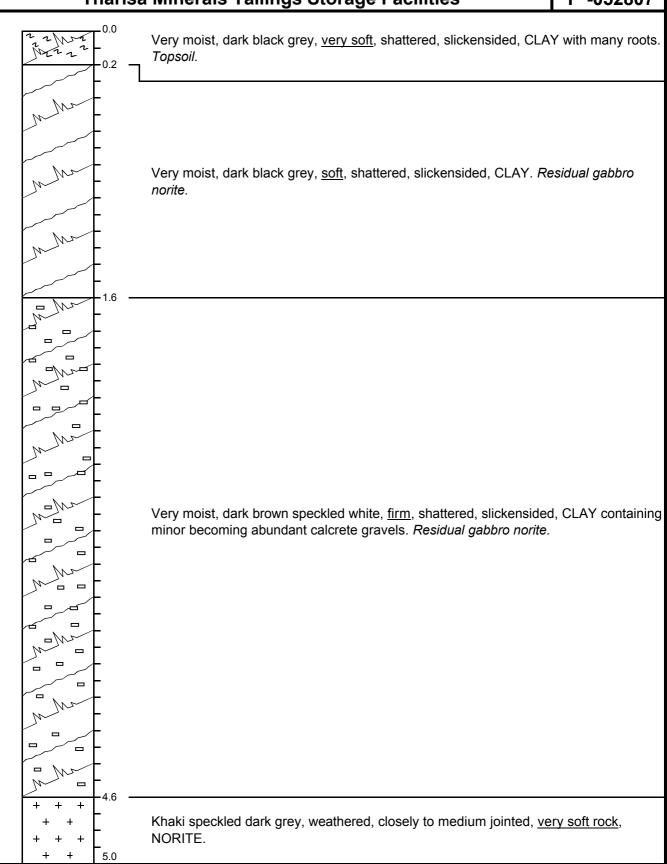
Sheet 1 of 1



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PROFILE SHEET TP17 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -052807



Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 04/04/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

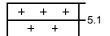
☐ Bulk sample ☐ In-situ test



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PROFILE SHEET	TP17 cont
Epoch Resources (Pty) Ltd.	X 2848362
Tharisa Minerals Tailings Storage Facilities	Y -052807



NOTES:

- 1. Bottom of hole at 5,1 m. Refusal on very soft to soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 04/04/2011

Water seepageStanding water

Undisturbed sampleDisturbed sample

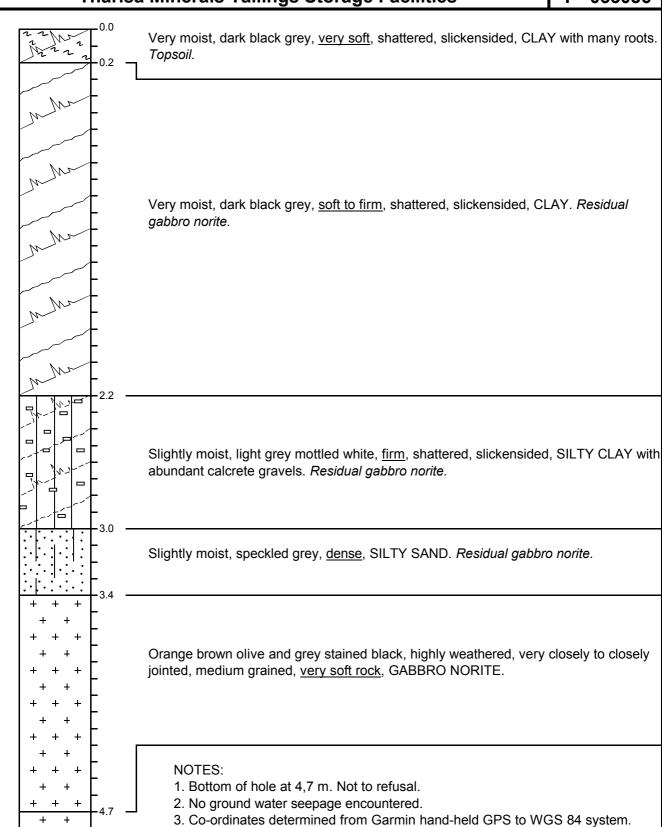
∃ Bulk sample
 ☐ In-situ test



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PROFILE SHEET TP18 Epoch Resources (Pty) Ltd. X 2848229 **Tharisa Minerals Tailings Storage Facilities** -053036



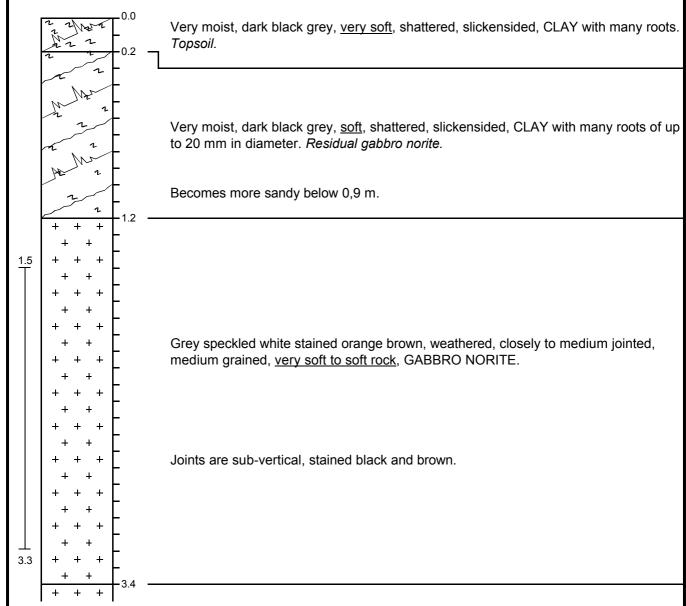
Contractor: 0 Profiled by: MC Shuping Date profiled: 04/04/2011 Machine: 0 Water seepage Undisturbed sample Ref: 0996/g Bulk sample Standing water Disturbed sample In-situ test Sheet 1 of 1



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PROFILE SHEET TP19 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -052755



NOTES:

- 1. Bottom of hole at 3,4 m. Refusal on very soft to soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Bulk sample taken from 1,5 to 3,3 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping
Machine: 0 Date profiled: 04/04/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample

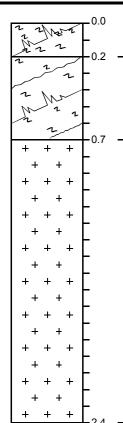
Ref: 0996/g



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PROFILE SHEET	TP20
Epoch Resources (Pty) Ltd.	X 2847993
Tharisa Minerals Tailings Storage Facilities	Y -052959



Very moist, dark black grey, <u>very soft</u>, shattered, slickensided, CLAY with many roots. *Topsoil*.

Very moist, dark black grey, <u>soft</u>, shattered, slickensided, CLAY with roots. *Residual gabbro norite*.

Grey and dark grey stained brown speckled white, weathered, very closely to medium jointed, medium grained, very soft to soft rock, GABBRO NORITE.

Joints are sub-vertical and horizontal, stained brown

NOTES:

- 1. Bottom of hole at 2,4 m. Partial refusal on soft rock GABBRO NORITE.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: 0 Profiled by: MC Shuping Machine: 0 Date profiled: 04/04/2011

✓ Water seepage Undisturbed sample ☐ Bulk sample Ref: 099

Water seepageStanding water

Disturbed sample

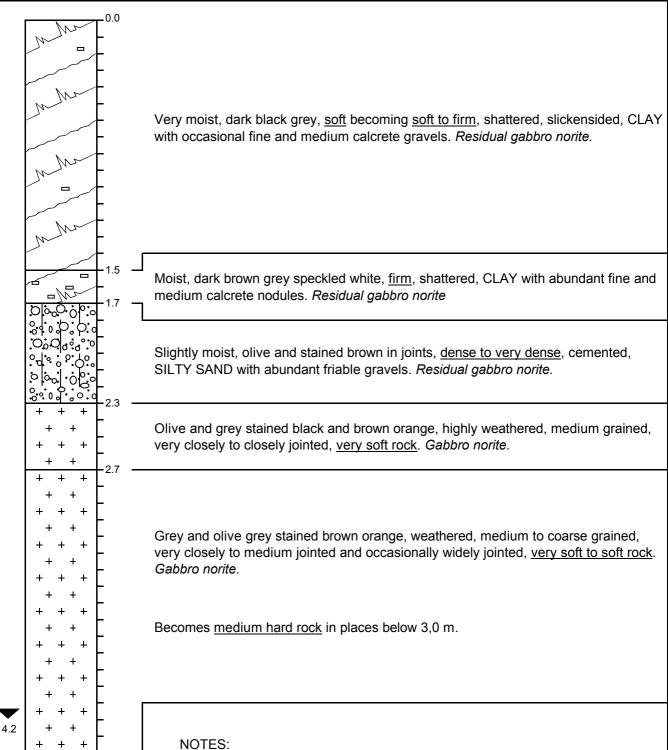
☐ Bulk sample☐ In-situ test



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PROFILE SHEET	TP21
Epoch Resources (Pty) Ltd.	X 2848169
Tharisa Minerals Tailings Storage Facilities	Y -050737



- 1. Bottom of hole at 4,6 m. Refusal on soft to medium hard rock gabbro norite.
- 2. Ground water seepage encountered below 4,2 m. Water standing at 4,2 m after 24 hours.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Water seepage
Standing water

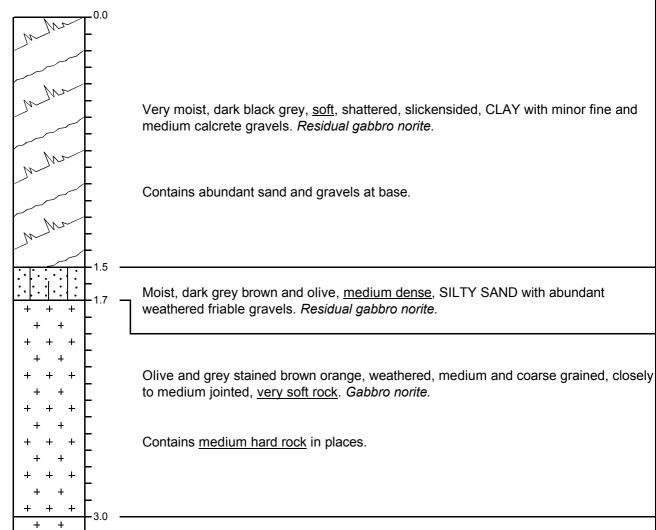
Disturbed sample



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PROFILE SHEET	TP22
Epoch Resources (Pty) Ltd.	X 2848234
Tharisa Minerals Tailings Storage Facilities	Y -051013



NOTES:

- 1. Bottom of hole at 3,0 m. Refusal on soft to medium hard rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Profiled by: MC Shuping
Date profiled: 13/06/2011

✓ Water seepage

Undisturbed sample

Standing water

Disturbed sample

In-situ test

Profiled by: MC Shuping
Date profiled: 13/06/2011

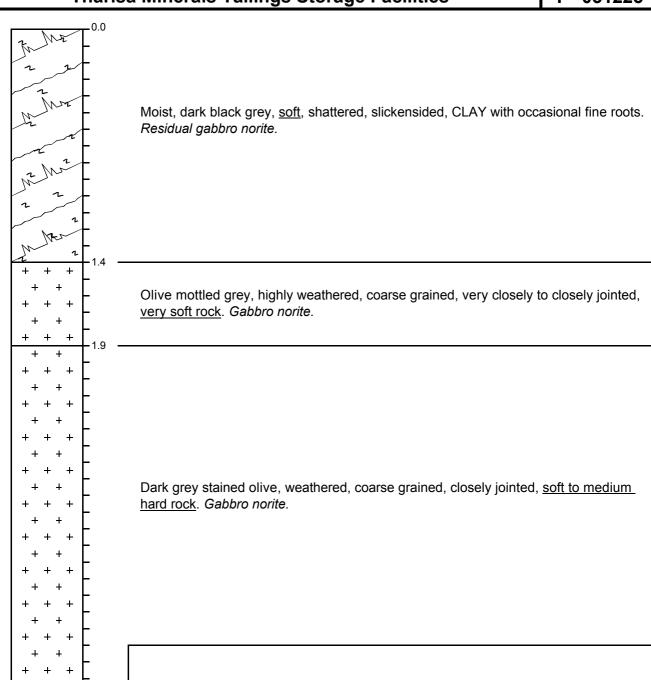
Ref: 0996/g



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PROFILE SHEET	TP23
Epoch Resources (Pty) Ltd.	X 2848282
Tharisa Minerals Tailings Storage Facilities	Y -051225



NOTES:

- 1. Bottom of hole at 4,4 m. Refusal on soft to medium hard rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Profiled by: MC Shuping
Date profiled: 13/06/2011

✓ Water seepage

Undisturbed sample

Standing water

Disturbed sample

In-situ test

Profiled by: MC Shuping
Date profiled: 13/06/2011

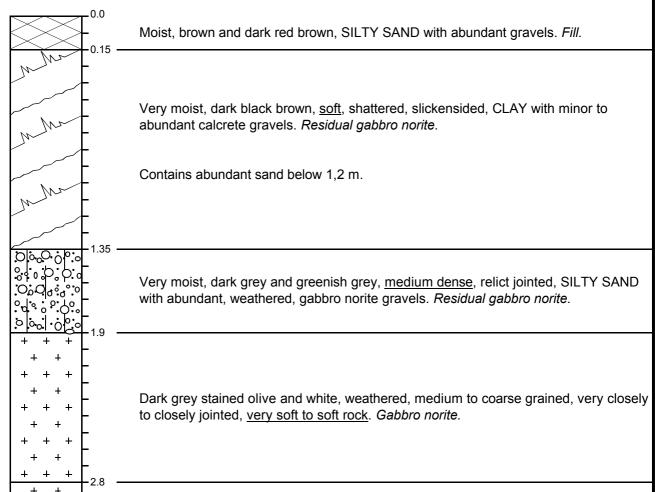
Ref: 0996/g



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PROFILE SHEET	TP24
Epoch Resources (Pty) Ltd.	X 2848229
Tharisa Minerals Tailings Storage Facilities	Y -051294



NOTES:

- 1. Bottom of hole at 2,8 m. Refusal on soft rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Profiled by: MC Shuping
Date profiled: 13/06/2011

✓ Water seepage

Undisturbed sample

Standing water

Disturbed sample

In-situ test

Profiled by: MC Shuping
Date profiled: 13/06/2011

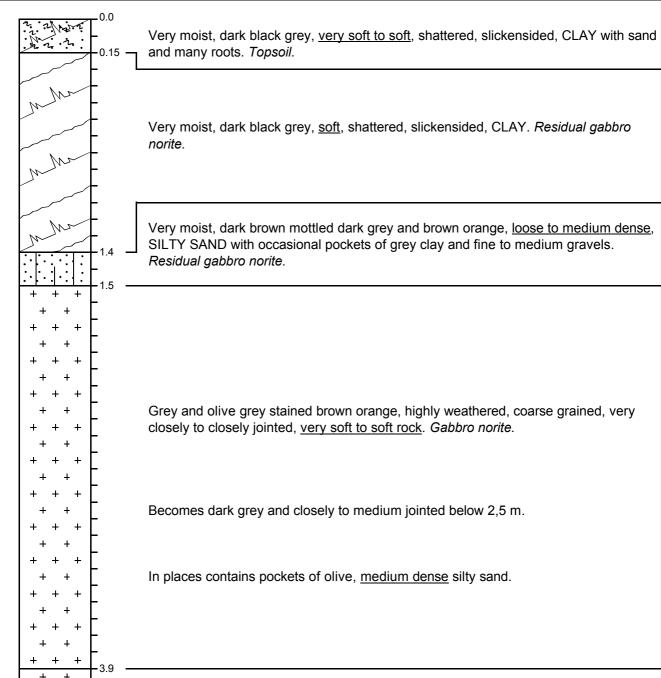
Ref: 0996/g



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PROFILE SHEET	TP25
Epoch Resources (Pty) Ltd.	X 2848378
Tharisa Minerals Tailings Storage Facilities	Y -051341



NOTES:

- 1. Bottom of hole at 3,9 m. Refusal on soft rock gabbro norite.
- 2. Very slow ground water seepage encountered below 1,0 m.
- 3. Excavated walls are highly collapsible up to a depth of 1,5 m.
- 4. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Water seepage

Undisturbed sample

Standing water

Profiled by: MC Shuping
Date profiled: 13/06/2011

Ref: 0996/g

In-situ test

Profiled by: MC Shuping
Date profiled: 13/06/2011

Ref: 0996/g

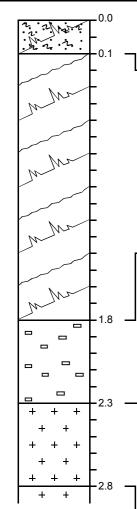
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PROFILE SHEET	TP26
Epoch Resources (Pty) Ltd.	X 2848764
Tharisa Minerals Tailings Storage Facilities	Y -051341



Very moist, dark black grey, soft, shattered, CLAY with sand and many roots. Topsoil.

Very moist, dark black grey becoming dark brown below 1,5 m, <u>soft</u>, shattered, slickensided, CLAY. *Residual gabbro norite*.

Closely packed, sub-angular and sub-rounded, fine and medium, calcrete and gabbro norite GRAVELS and calcrete powder in a matrix of slightly moist, orange brown silty sand. *Calcrete*.

Contains up to 50 mm thick pockets of brown clay and fine ferruginous nodules.

Light grey speckled dark grey stained brown orange, weathered, medium to coarse grained, very closely to closely jointed, <u>very soft rock</u>. *Gabbro norite*.

Becomes closely to medium jointed and soft rock below 2,5 m.

NOTES:

- 1. Bottom of hole at 2,8 m. Refusal on soft rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Profiled by: MC Shuping
Date profiled: 13/06/2011

✓ Water seepage

Undisturbed sample

Bulk sample

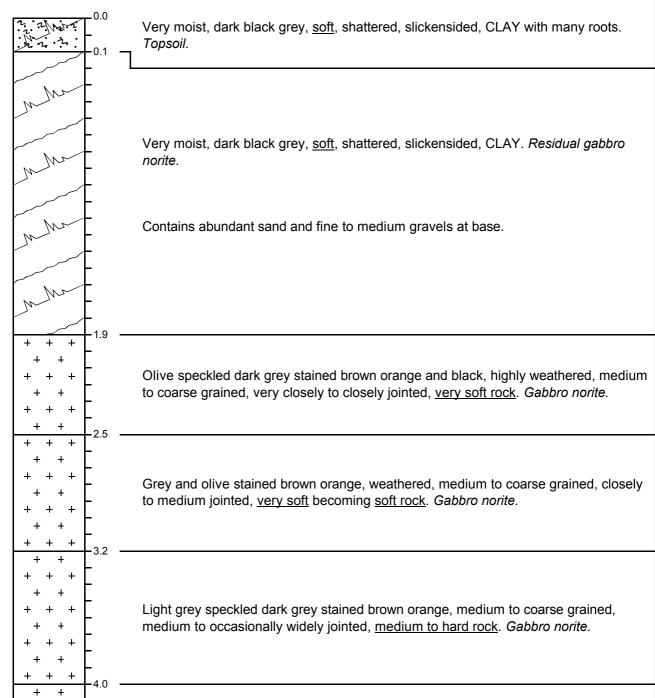
Ref: 0996/g



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PROFILE SHEET TP27 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -051068



NOTES:

- 1. Bottom of hole at 4,0 m. Refusal on medium hard rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Water seepage

Undisturbed sample

Standing water

Profiled by: MC Shuping
Date profiled: 13/06/2011

Ref: 0996/g

In-situ test

Ref: 0996/g

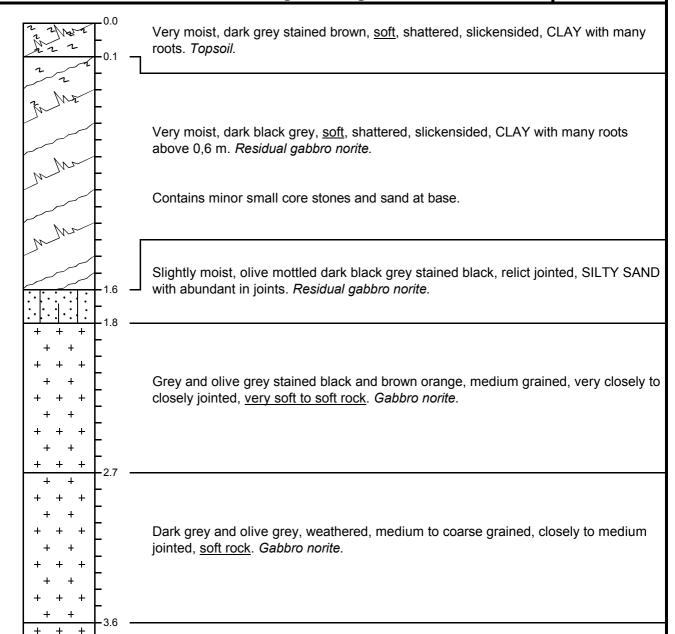
Sheet 1 of 1



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PROFILE SHEET TP28 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -051039



NOTES:

- 1. Bottom of hole at 3,6 m. Refusal on soft to medium hard rock gabbro norite.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Profiled by: MC Shuping
Date profiled: 13/06/2011

Water seepage

Undisturbed sample

Standing water

Disturbed sample

In-situ test

Profiled by: MC Shuping
Date profiled: 13/06/2011

Ref: 0996/g

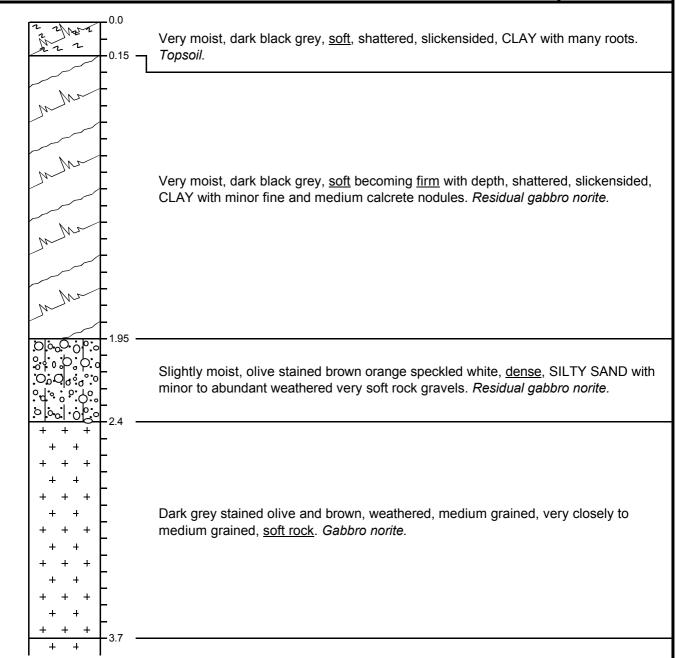
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PROFILE SHEET TP29 Epoch Resources (Pty) Ltd. Tharisa Minerals Tailings Storage Facilities Y -050923



NOTES:

- 1. Bottom of hole at 3,7 m. Not to refusal.
- 2. No ground water seepage encountered.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.
- 1. Bottom of hole at 1,7 m. Not to refusal.

Contractor: MCC

Machine: 0

Water seepage

Undisturbed sample

Standing water

Profiled by: MC Shuping
Date profiled: 13/06/2011

Ref: 0996/g

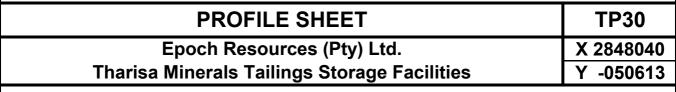
In-situ test

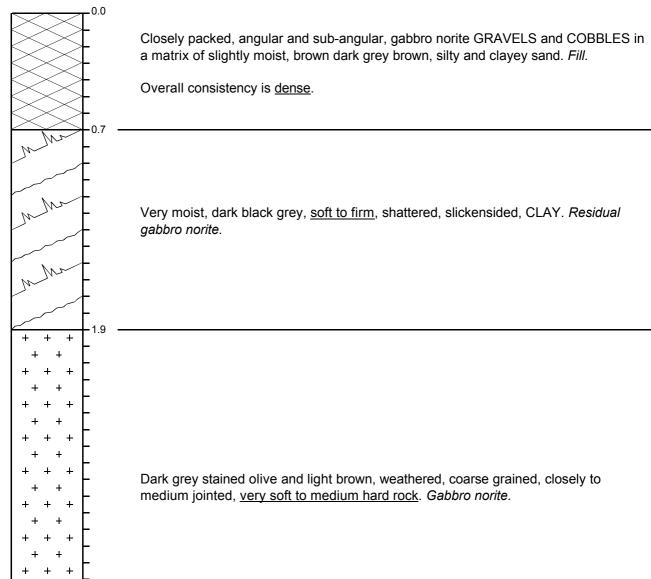
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NOTES:

3.6

- 1. Bottom of hole at 4,4 m. Refusal.
- 2. Ground water encountered below 3,6 m. Water standing at 3,6 m after 24 hours.
- 3. Co-ordinates determined from Garmin hand-held GPS to WGS 84 system.

Contractor: MCC

Machine: 0

Water seepage

✓ Standing water

Contractor: MCC

Profiled by: MC Shuping

Date profiled: 13/06/2011

Ref: 0996/g

In-situ test

Frofiled by: MC Shuping

Date profiled: 13/06/2011

Ref: 0996/g

Sheet 1 of 1

Inroads Consulting	Ref: 10151/g
APPEND	
APPEND	
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Geotechnical Investigation 14	Tharisa Tailings Storage Facility

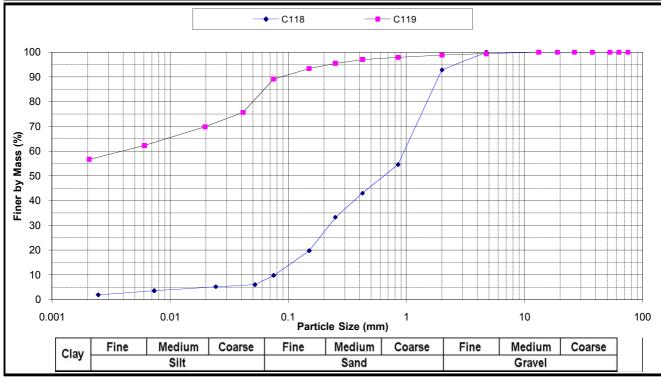
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Foundation Indicator Test Data

Project THARISA MINERALS							
Project No.	1039/F42/04	/2011		Date	19 May 2011		
Sample No.	C118	C119		Sample No.	C118	C119	
Field Ref. No.	TP 2	TP 4		%Gravel	7	1	
Depth	1.6 - 4.0	1.0 - 1.2		%Sand	85	15	
Sieve size	%Passing	% Passing	% Passing	%Silt	6	28	
75	100	100		%Clay	2	57	
63	100	100		NMC %	Not Tested	Not Tested	
53	100	100		Liquid Limit	NP	63	
37.5	100	100		Plasticity	NP	34	
26.5	100	100		Index	141		
19.0	100	100		Linear Shrink.	0.	17.5	
13.2	100	100		Overall P.I.	NP	33	
4.75	100	99		Grading	1.54	0.15	
2.00	93	99		Modulus	1.54	0.13	
0.85	55	98		H.R.B.	A-1-b (0)	A-7-6 (20)	
0.425	43	97		Unified	SP-SM	CH	
0.250	33	96		Weston swell			
0.150	20	93		(%) at 1 kPa			
0.075	10	89		Analysis as p	er method D4	22 of ASTM	of 1985
0.04	6	75		The results re	eported relate	only to the	
0.02	5	70		samples teste	ed.		
0.006	3	62		Documents may only be reproduced or			
0.002	2	57		published in their full context.			



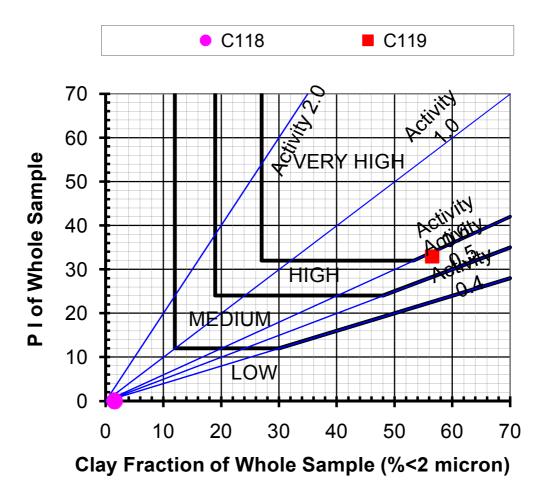
Remarks:

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Activity Diagram After D H van der Merwe



Plotted Values:

<u>Sample</u>	Clay Frac	<u>PI</u>
C118	1.6	#VALUE!
C119	56.5	32.9

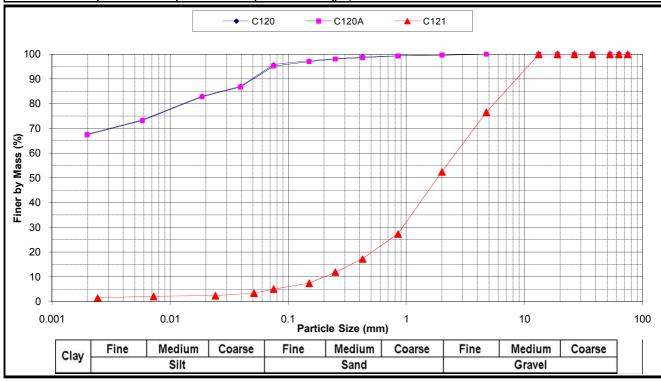
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Foundation Indicator Test Data

Project THARISA MINERALS							
Project No.	1039/F42/04	/2011		Date	19 May 2011		
Sample No.	C120	C120A	C121	Sample No.	C120	C120A	C121
Field Ref. No.	TP 6	TP 6	TP 6	%Gravel	0	0	48
Depth	1.0 - 1.2	1.0 - 1.2	4.7 - 5.1	%Sand	7	7	48
Sieve size	%Passing	% Passing	% Passing	%Silt	25	25	3
75	100	100	100	%Clay	68	67	1
63	100	100	100	NMC %	Not Tested	Not Tested	Not Tested
53	100	100	100	Liquid Limit	60	63	SP
37.5	100	100	100	Plasticity	32	34	SP
26.5	100	100	100	Index	32	34	SP SP
19.0	100	100	100	Linear Shrink.	15.	15.	1.
13.2	100	100	100	Overall P.I.	31	33	SP
4.75	100	100	77	Grading	0.06	0.06	2.25
2.00	100	100	52	Modulus	0.00	0.06	2.20
0.85	99	99	27	H.R.B.	A-7-6 (20)	A-7-6 (20)	A-1-b (0)
0.425	99	99	17	Unified	CH	CH	SW-SM
0.250	98	98	12	Weston swell			
0.150	97	97	7	(%) at 1 kPa			
0.075	96	95	5	Analysis as p	er method D4	22 of ASTM	of 1985
0.04	87	87	3	The results re	eported relate	only to the	
0.02	83	83	2	samples tested.			
0.006	74	73	2	Documents may only be reproduced or			
0.002	68	67	1	published in their full context.			



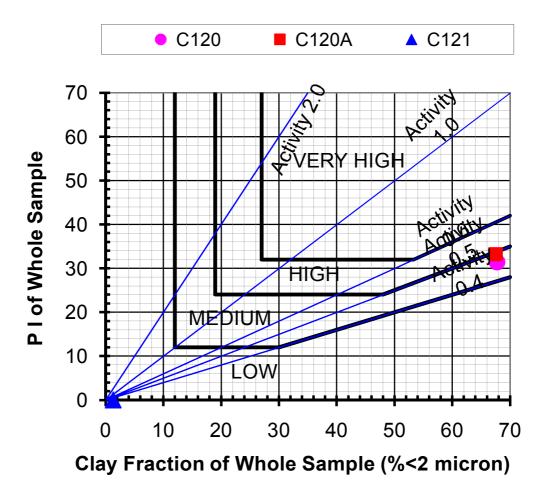
Remarks:

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Activity Diagram After D H van der Merwe



Plotted Values:

<u>Sample</u>	Clay Frac	<u>PI</u>
C120	67.7	31.4
C120A	67.5	33.2
C121	1.3	#VALUE!

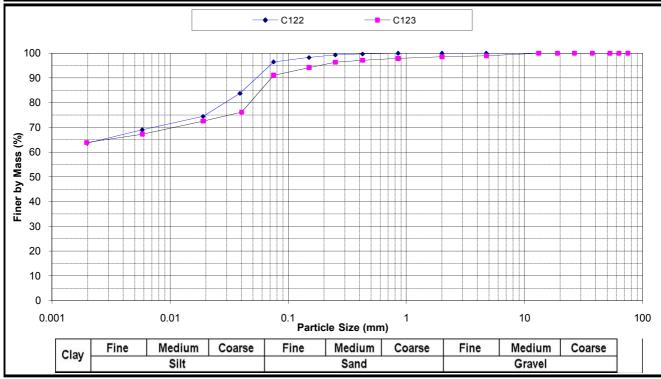
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Foundation Indicator Test Data

Project	THARISA M	INERALS					
Project No.	1039/F42/04	/2011		Date	1 June 2011		
Sample No.	C122	C123		Sample No.	C122	C123	
Field Ref. No.	TP 9	TP 10		%Gravel	0	1	
Depth	4.5 - 4.7	4.0 - 4.2		%Sand	8	13	
Sieve size	%Passing	% Passing	% Passing	%Silt	28	22	
75	100	100		%Clay	64	64	
63	100	100		NMC %	Not Tested	Not Tested	
53	100	100		Liquid Limit	105	98	
37.5	100	100		Plasticity	72	64	
26.5	100	100		Index	12	04	
19.0	100	100		Linear Shrink.	22.	20.	
13.2	100	100		Overall P.I.	72	62	
4.75	100	99		Grading	0.04	0.13	
2.00	100	99		Modulus	0.04	0.13	
0.85	100	98		H.R.B.	A-7-5 (20)	A-7-5 (20)	
0.425	100	97		Unified	CH	CH	
0.250	99	96		Weston swell			
0.150	98	94		(%) at 1 kPa			
0.075	97	91		Analysis as per method D422 of ASTM of 1985			
0.04	84	76		The results reported relate only to the			
0.02	75	73		samples teste	ed.		
0.006	69	67		Documents m	nay only be re	produced or	
0.002	64	64		published in their full context.			



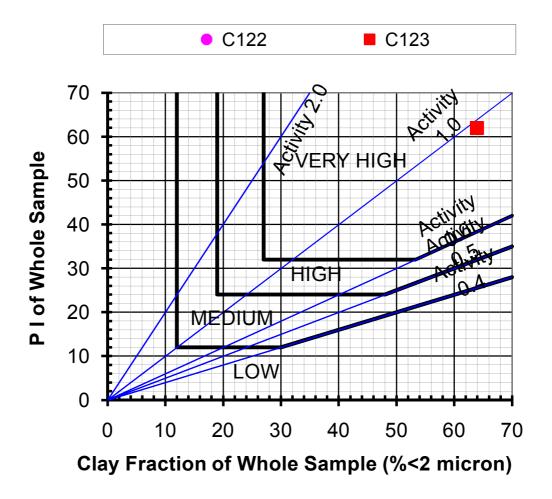
Remarks:

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Activity Diagram After D H van der Merwe



Plotted Values:

<u>Sample</u>	Clay Frac	<u>PI</u>
C122	63.7	71.9
C123	63.9	61.9

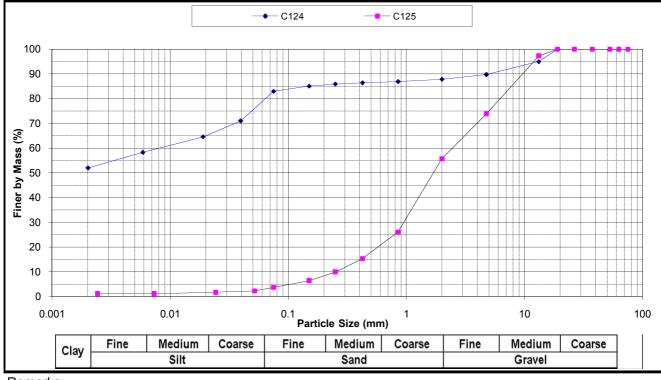
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Foundation Indicator Test Data

Project	THARISA M	INERALS					
Project No.	1039/F42/04/2011			Date	1 June 2011		
Sample No.	C124	C125		Sample No.	C124	C125	
Field Ref. No.	TP 14	TP 19		%Gravel	12	44	
Depth	4.0 - 4.2	1.2 - 3.4		%Sand	9	53	
Sieve size	%Passing	% Passing	% Passing	%Silt	27	2	
75	100	100		%Clay	52	1	
63	100	100		NMC %	Not Tested	Not Tested	
53	100	100		Liquid Limit	93	NP	
37.5	100	100		Plasticity	58	NP	
26.5	100	100		Index	30	IVI	
19.0	100	100		Linear Shrink.	20.5	0.	
13.2	95	97		Overall P.I.	50	NP	
4.75	90	74		Grading	0.43	2.25	
2.00	88	56		Modulus	0.43	2.25	
0.85	87	26		H.R.B.	A-7-5 (20)	A-1-b (0)	
0.425	86	15		Unified	CH	SW	
0.250	86	10		Weston swell			
0.150	85	6		(%) at 1 kPa			
0.075	83	4		Analysis as p	er method D4	22 of ASTM	of 1985
0.04	71	2		The results re	eported relate	only to the	
0.02	65	2		samples teste	ed.		
0.006	58	1		Documents m	nay only be re	produced or	
0.002	52	1		published in t	heir full conte	ext.	

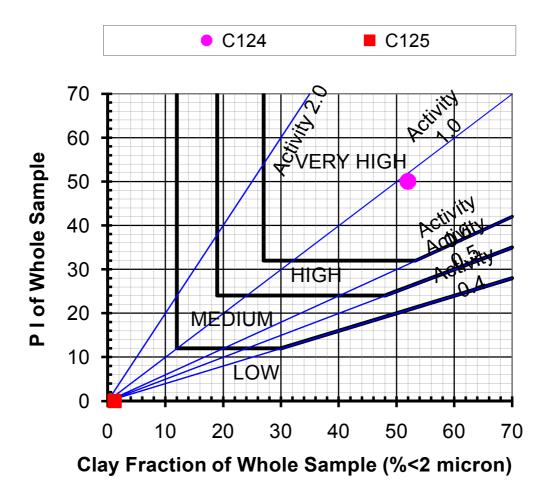


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Activity Diagram After D H van der Merwe



Plotted Values:

<u>Sample</u>	Clay Frac	<u>PI</u>
C124	52.0	50.0
C125	1.2	#VALUE!

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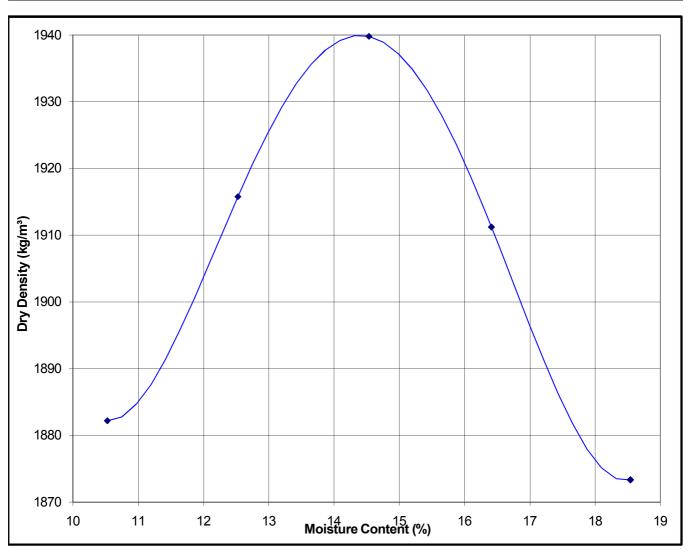
Moisture Density Relationship

Project:	THARISA MINERALS		
Project No.:	1039/F42/04/2011	Date:	20 April 2011
Field Reference:	TP 2	Laboratory Ref.:	C118
Depth (m):	1.6 - 4.0	Remarks:	Untreated
Description:	-		

Compactive Effort: STD PROCTOR

Percent Water Content (%):	16.4	18.5	14.5	12.5	10.5		
Dry Density (kg/m³):	1911	1873	1940	1916	1882		

Maximum Dry Density: 1940 kg/m³ Optimum Moisture Content: 14.3 %



Analysis according to Method A7 of TMH1 of 1986.

The results relate only to the samples tested.

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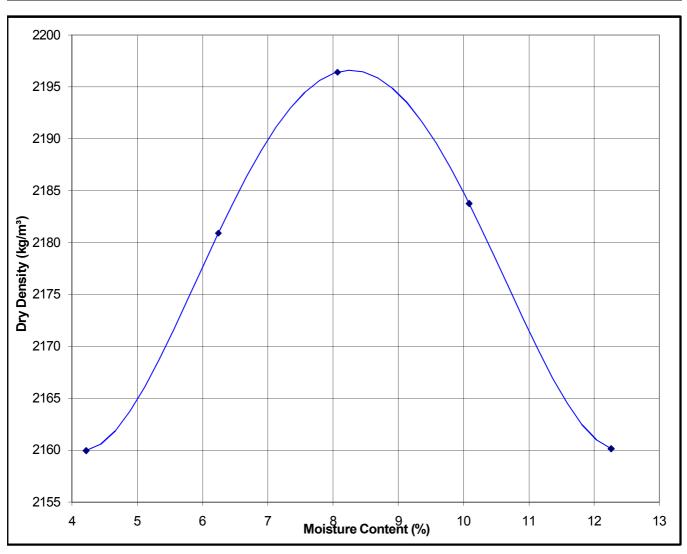
Moisture Density Relationship

Project:	THARISA MINERALS		
Project No.:	1039/F42/04/2011	Date:	20 April 2011
Field Reference:	TP 6	Laboratory Ref.:	C121
Depth (m):	4.7 - 5.1	Remarks:	Untreated
Description:	-		

Compactive Effort: STD PROCTOR

Percent Water Content (%):	8.1	10.1	6.2	4.2	12.3		
Dry Density (kg/m³):	2196	2184	2181	2160	2160		

Maximum Dry Density: 2197 kg/m³ Optimum Moisture Content: 8.2 %



Analysis according to Method A7 of TMH1 of 1986.

The results relate only to the samples tested.

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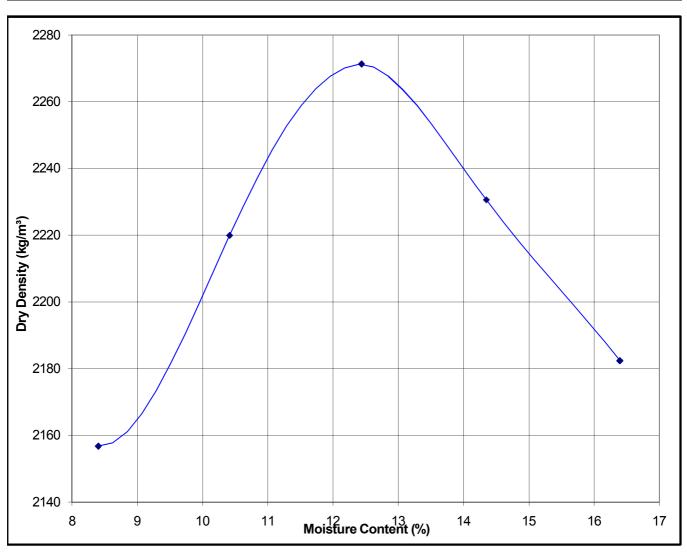
Moisture Density Relationship

Project:	THARISA MINERALS		
Project No.:	1039/F42/04/2011	Date:	20 April 2011
Field Reference:	TP 19	Laboratory Ref.:	C125
Depth (m):	1.2 - 3.4	Remarks:	Untreated
Description:	-		

Compactive Effort: STD PROCTOR

Percent Water Content (%):	10.4	12.4	14.3	8.4	16.4		
Dry Density (kg/m³):	2220	2271	2231	2157	2182		

Maximum Dry Density: 2271 kg/m³ Optimum Moisture Content: 12.4 %



Analysis according to Method A7 of TMH1 of 1986.

The results relate only to the samples tested.

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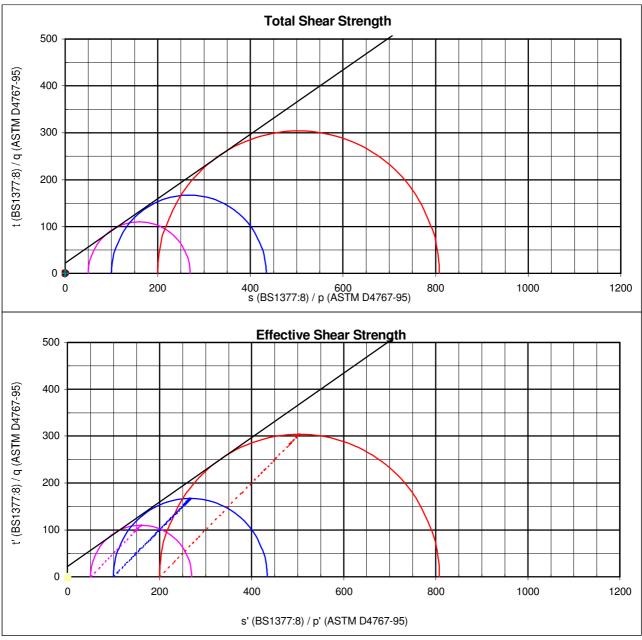
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

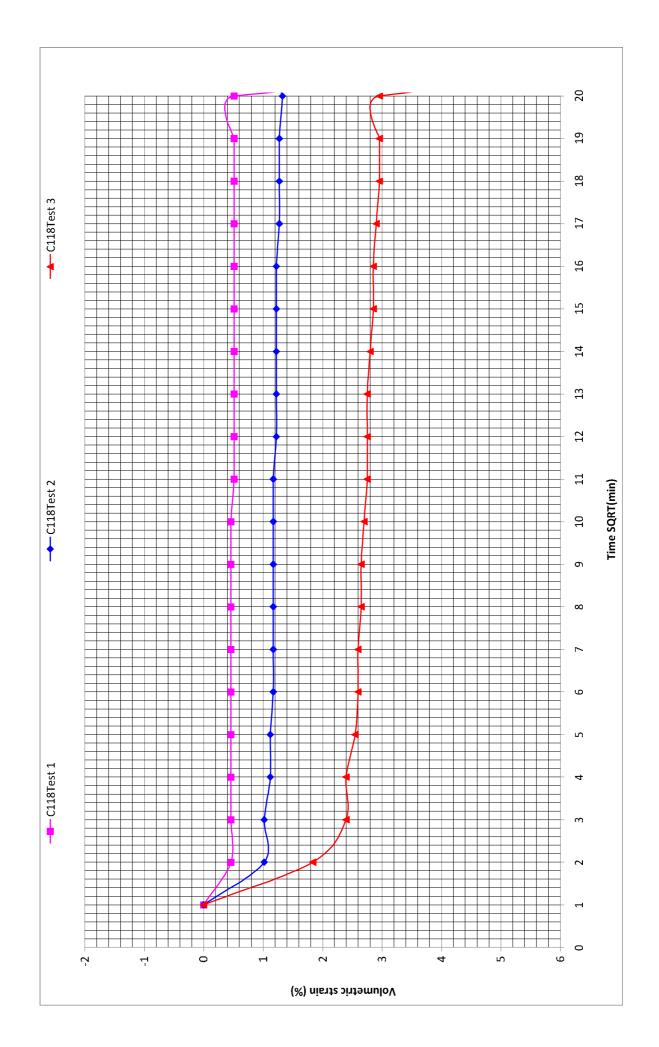
Project:	THARISA MINERALS	Date Tested:	01/06/2011
Proj.No.:	1039/F42/04/2011	Laboratory Number:	C118
Field Sample Reference:	TP 2	Depth (m):	1.6 - 4.0

Mohr Stress Circles

	COHESION (kPa)	FRICTION ANGLE
TOTAL STRESSES	22	35
EFFECTIVE STRESSES	22	35



A Consolidated Drained test on a remoulded sample tested saturated.



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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	01/06/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C118
Field Sample Number:	TP 2	Depth (m):	1.6 - 4.0

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks: A Consolidated Drained test on a remoulded sample tested saturated.

Test No. 1 **SATURATION DATA**

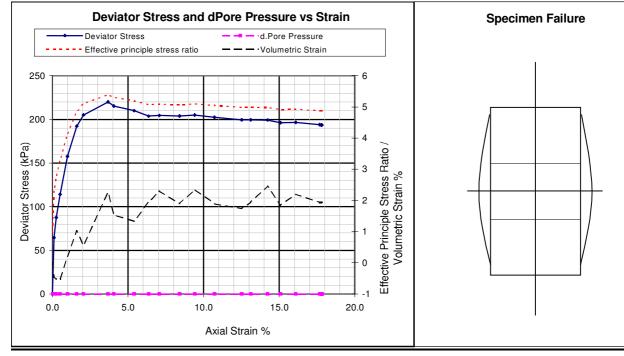
Saturation method:	Alternatin	g increments of cell- & back pressure	
Pressure increments app	lied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa):	550.0	Final back pressure (kPa): 540.0	Final B parameter: 0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa): 50.0		50.0	.0 t100 (minutes): 0			Side drains fitted: No		
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	14.6	1.820	0.6827	65	2.060
CONSOLIDATED	99.83	49.92	1956.83	19.1	1.829	0.6741	87	3.062 Determined
FINAL (After shear)	82.06	54.52	2334.27	17.9	1.865	0.6416	85	Determined
Initial pore pressure (kPa): 580.5 Final pore pressure (kPa): 541.4 Pore pressure dissipation: 97%						n: 97%		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	9					
Initial pore pressure (kPa): 5	41.4 l	nitial effective stress (kPa):	50.0			
Failure Criterion: Max	x. Deviator S	tress				
Axial strain (%):	3.68	Volumetr	ic strain (%):	2.25		
Deviator stress (kPa):	219.9			Principle St	resses (kPa)	
Excess pore pressure (kPa):	0.0		O 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	5.399		269.9	269.9	50.0	50.0



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Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	1900/01/00
Batch No.:	1039/F42/04/2011	Laboratory Number:	C118
Field Sample Number:	TP 2	Depth (m):	1.6 - 4.0

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks: A Consolidated Drained test on a remoulded sample tested saturated.

SATURATION DATA Test No. 2

Saturation method:	Alternatin	g increments of cell- & back pressure	
Pressure increments appl	lied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa):	550.0	Final back pressure (kPa): 540.0	Final B parameter: 0.99

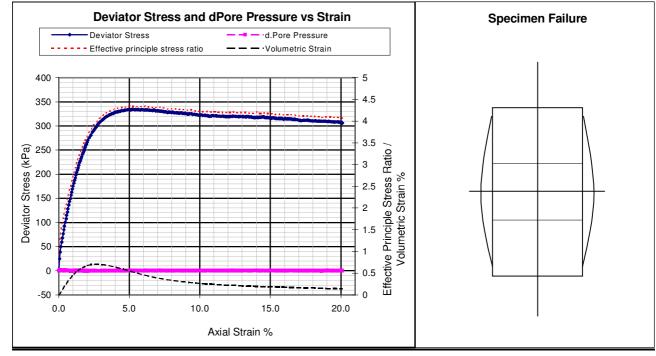
CONSOLIDATION DATA

Effective cons. Stress (kPa):		100.0 t100 (minutes): 0			Side drains fitted: No			
Height		Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	15.6	1.813	0.6888	69	0.000
CONSOLIDATED	99.56	49.78	1946.16	20.0	1.837	0.6665	92	3.062 Determined
FINAL (After shear)	79.58	55.64	2431.52	19.7	1.840	0.6642	91	Determined
Initial pore pressure (kPa): 637.1 Final pore pressure (kPa): 545.1 Pore pressure dissipation: 95%					n: 95%			
*: Measured dimensions; a	all other din	nensions are	e calculate	d.				

SHEAR DATA

Rate of strain (%/hour):	9						
Initial pore pressure (kPa):	543.1	Initial effectiv	ve stress (kPa):	100.0			
Failure Criterion:	Max. Deviate	or Stress					
Axial strain (%):	4.	96	Volumetrio	c strain (%):	0.56		
Deviator stress (kPa):	33	34.2			Principle St	resses (kPa)	
Excess pore pressure (kPa)): 0.	2		σ 1	σ 1'	O 3	ග ³'
Effective principle stress rat	io: 4.	349		434.2	433.9	100.0	99.8

Deviator stress corrections: Membrane correction: 1.14 kPa



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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	1900/01/00
Batch No.:	1039/F42/04/2011	Laboratory Number:	C118
Field Sample Number:	TP 2	Depth (m):	1.6 - 4.0

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,8

Remarks: A Consolidated Drained test on a remoulded sample tested saturated.

Test No. 3 **SATURATION DATA**

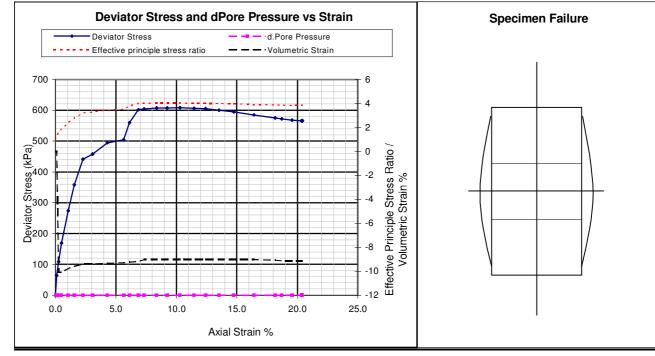
Saturation method:	Alternatin	g increments of cell- & back pressure	
Pressure increments app	lied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa):	550.0	Final back pressure (kPa): 540.0	Final B parameter: 0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa):		200.0 t100 (minutes): 0			Side drains fitted: No			
Height		Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	15.6	1.803	0.6979	69	0.000
CONSOLIDATED	99.02	49.51	1924.83	14.9	1.858	0.6477	70	3.062 Determined
FINAL (After shear)	78.84	57.96	2638.30	19.8	1.702	0.7986	76	Determined
Initial pore pressure (kPa): 733.5 Final pore pressure (kPa): 544.1 Pore pressure dissipation: 989					n: 98%			
*: Measured dimensions; a	*: Measured dimensions; all other dimensions are calculated.							

SHEAR DATA

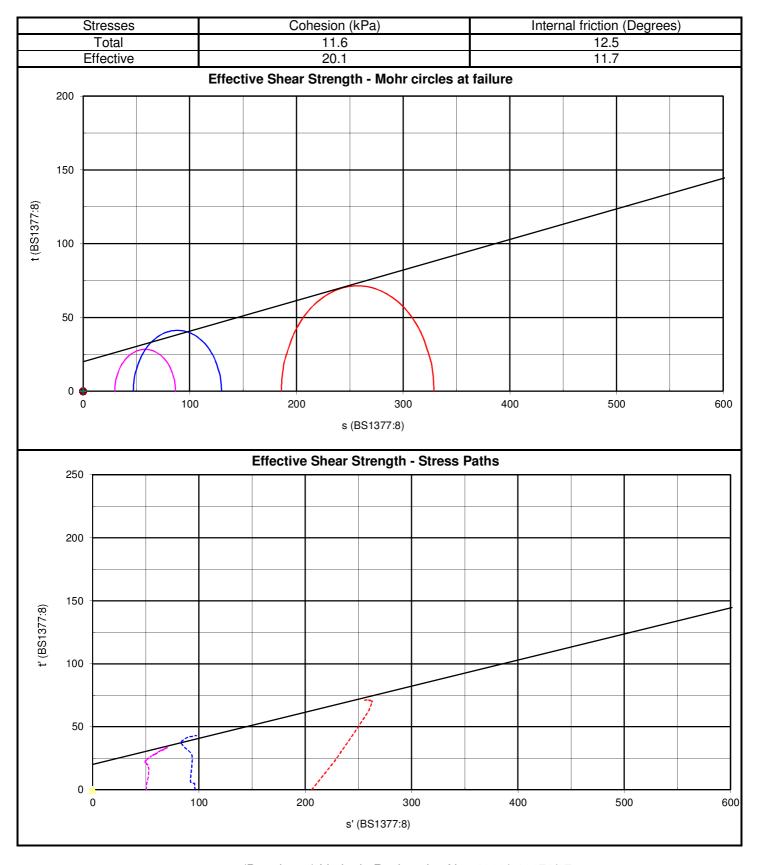
Rate of strain (%/hour):	9						
Initial pore pressure (kPa):	544.1	Initial effecti	ve stress (kPa):	200.0			
Failure Criterion:	Max. Deviato	r Stress					
Axial strain (%):	10.	30	Volumetrio	c strain (%):	-9.03		
Deviator stress (kPa):	607	7.9			Principle St	resses (kPa)	
Excess pore pressure (kPa)): 0.0			σ 1	σ 1'	O 3	O 3'
Effective principle stress rat	io: 4.0	40		807.9	807.9	200.0	200.0



Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	17/06/2011
Proj.No.:	F42/04/11	Laboratory Number:	C119
Field Sample Reference:	TP 4	Depth (m):	1.0-1.2

Effective Shear Strength



Triaxial	Compression	Test Results
----------	-------------	--------------

Project:	THARISA MINERALS	Date Tested:	17/06/2011
Batch No.:	F42/04/11	Laboratory Number:	C119
Field Sample Number:	TP 4	Depth (m):	1.0-1.2

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 1

Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.00

CONSOLIDATION DATA

Effective cons. Stress (kPa):		50.7 t100 (minutes): 10.24			Side drains fitted: No		No	
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*100	*50	1963.50	35.6	1240	1.1369	83	0.05
CONSOLIDATED	99.49	49.74	1943.50	39.4	1259	1.1042	95	2.65 Determined
FINAL (After shear)	79.50	55.65	2432.06	39.4	1259	1.1043	95	Determined
Initial pore pressure (kPa):	383.7	Final pore pressure (kPa): 345.1			345.1	Pore pressi	ure dissipatio	n: 88%
*: Measured dimensions; a	*: Measured dimensions; all other dimensions are calculated.							

SHEAR DATA

Rate of strain (%/hour):	9						
Initial pore pressure (kPa):	346.3	Initial effective stre	ess (kPa): 5	50.7			
Parameters at failure:							
Failure Criterion:	Max. Effective Pr	rinciple Stress Ratio					
Axial strain (%):	8.49)					
Deviator stress (kPa):	56.9)			Principle Str	esses (kPa)	
Excess pore pressure (kPa)	: 20.8	3		σ1	σ1'	O 3	σ3'
Effective principle stress rati	io: 2.90)7		107.5	86.7	50.7	29.8

Deviator stress corrections: Membrane correction: 1.1 kPa **Deviator Stress and dPore Pressure vs Strain Specimen Failure Deviator Stress** d.Pore Pressure Effective principle stress ratio 80 3.5 70 3 60 Effective Principle Stress Ratio Deviator Stress (kPa) 50 2.5 40 30 20 1.5 10 0 0.0 5.0 10.0 15.0 20.0 25.0 Axial Strain %

Triaxial	Com	pression	Test	Results
----------	-----	----------	------	---------

Project:	THARISA MINERALS	Date Tested:	17/06/2011
Batch No.:	F42/04/11	Laboratory Number:	C119
Field Sample Number:	TP 4	Depth (m):	1.0-1.2

Remarks:	A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 2

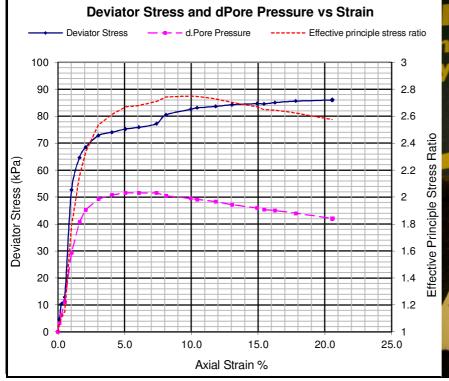
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.00

CONSOLIDATION DATA

Effective cons. Stress (kPa):		96.7 t100 (minutes): 7.02			Side drains fitted: No			
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm ²	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*100	*50	1963.50	33.7	1259	1.1040	81	0.05
CONSOLIDATED	98.74	49.37	1914.16	37.0	1309	1.0247	96	2.65 Determined
FINAL (After shear)	78.44	55.39	2409.52	37.0	1308	1.0254	96	Determined
Initial pore pressure (kPa):	434.0	Final pore pressure (kPa): 345.0			345.0	Pore press	ure dissipati	on: 95%
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	9					
Initial pore pressure (kPa):	350.3 Initi	al effective stress (kPa):	96.7			
Parameters at failure:						
Failure Criterion: Ma	ax. Effective Principl	le Stress Ratio				
Axial strain (%):	9.97					
Deviator stress (kPa):	82.6			Principle Str	resses (kPa))
Excess pore pressure (kPa):	49.5		σ 1	σ1'	O 3	O 3'
Effective principle stress ratio	2.750		179.3	129.8	96.7	47.2





Triaxial	Com	pression	Test	Results
----------	-----	----------	------	---------

Project:	THARISA MINERALS	Date Tested:	17/06/2011
Batch No.:	F42/04/11	Laboratory Number:	C119
Field Sample Number:	TP 4	Depth (m):	1.0-1.2

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 3

Saturation method: Alternating	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.00

CONSOLIDATION DATA

Effective cons. Stress (kPa):		206.0 t100 (minutes): 31.4			Side drains fitted: No			
Height		Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm ²	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*74	*38	1134.11	33.7	1099	1.4121	63	0.05
CONSOLIDATED	71.41	36.65	1054.84	37.1	1227	1.1592	85	2.65 Determined
FINAL (After shear)	65.41	38.29	1151.60	37.1	1224	1.1651	84	Determined
Initial pore pressure (kPa): 532.0 Final pore pressure (kPa): 343.0 Pore pressure dissipation: 98%					n: 98%			
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

9							
341.0	Initial	effective stres	s (kPa): 2	206.0			
Max. Deviato	r Stress						
7	.09						
1-	42.9				Principle Str	esses (kPa)	
): 2	0.0			σ 1	σ1'	O 3	G 3'
io: 1	.768			348.8	328.8	206.0	186.0
)	341.0 Max. Deviato 7 1 : 2	341.0 Initial Max. Deviator Stress 7.09 142.9 : 20.0	341.0 Initial effective stres Max. Deviator Stress 7.09 142.9 : 20.0	341.0 Initial effective stress (kPa): 2 Max. Deviator Stress 7.09 142.9 : 20.0	341.0 Initial effective stress (kPa): 206.0 Max. Deviator Stress 7.09 142.9 : 20.0 σ1	341.0 Initial effective stress (kPa): 206.0 Max. Deviator Stress 7.09 142.9 Principle Str :: 20.0 σ1 σ1'	341.0 Initial effective stress (kPa): 206.0 Max. Deviator Stress 7.09 142.9 Principle Stresses (kPa) :: 20.0 σ1 σ1' σ3

Deviator stress corrections: Membrane correction: 0.1 kPa **Deviator Stress and dPore Pressure vs Strain** Specimen Failure **Deviator Stress** ----- Effective principle stress ratio 160 1.9 1.8 140 1.7 120 1.6 Deviator Stress (kPa) 100 1.5 80 Principle 60 1.3 40 1.2 20 1.1 0 0.0 2.0 4.0 6.0 8.0 10.0 Axial Strain %

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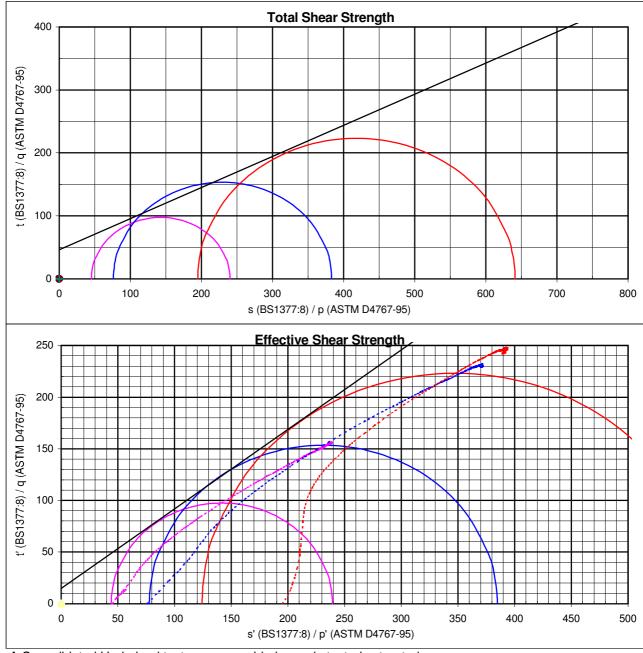
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Triaxial Compression Test Results

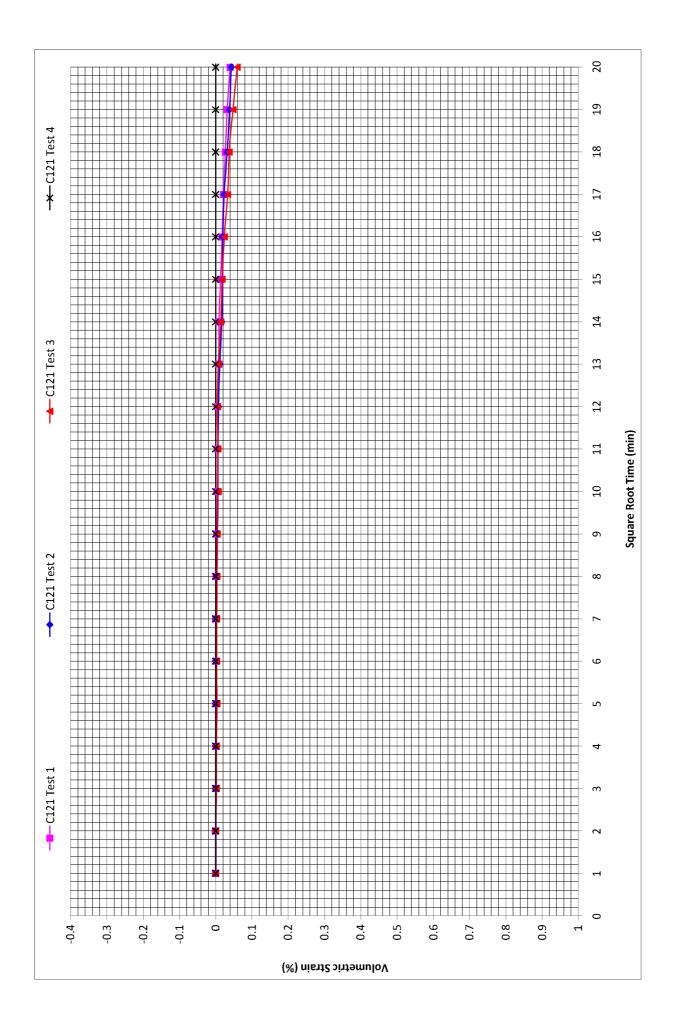
Project:	THARISA MINERALS	Date Tested:	31/05/2011
Proj.No.:	1039/F42/04/2011	Laboratory Number:	C121
Field Sample Reference:	TP 6	Depth (m):	4.7 - 5.1

Mohr Stress Circles

	COHESION (kPa)	FRICTION ANGLE
TOTAL STRESSES	46	26
EFFECTIVE STRESSES	15	38



A Consolidated Undrained test on a remoulded sample tested saturated.



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Triaxial Com	pression	Test	Resul	ts

Project:	THARISA MINERALS	Date Tested:	31/05/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C121
Field Sample Number:	TP 6	Depth (m):	4.7 - 5.1

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

	, , ,	
Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.	

SATURATION DATA Test No. 1

Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.99

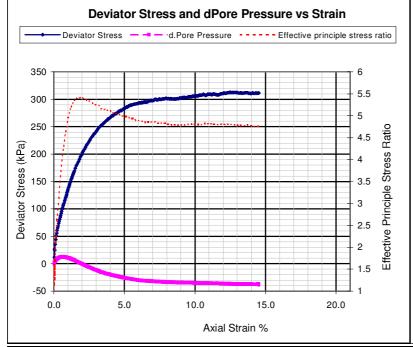
CONSOLIDATION DATA

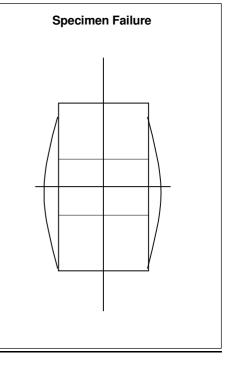
Effective cons. Stress (kPa): 45.3			t100 (minutes): 30			Side drains fitted: No		
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	9.5	2.051	0.6418	50	3.368
CONSOLIDATED	99.39	49.69	1939.50	10.7	2.090	0.6117	59	3.366 Determined
FINAL (After shear)	84.97	53.74	2268.59	10.7	2.090	0.6118	59	Determined
Initial pore pressure (kPa): 346.0 Final pore pressure (kPa): 346.0 Pore pressure dissipation: 0%						n: 0%		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	9					
Initial pore pressure (kPa): 34	45.6	Initial effective stress (kPa):	45.3			
Failure Oritorian May	. Effective l	Dringinla Ctropa Datia				
Failure Criterion: Max	t. Effective i	Principle Stress Ratio				
Axial strain at failure (%):	1.88					
Deviator stress (kPa):	195.1			Principle Str	resses (kPa)	
Excess pore pressure (kPa):	1.1		O 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	5.420)	240.4	239.3	45.3	44.1

Deviator stress corrections: Membrane correction: 1.14 kPa





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Triaxial Compression Test Results				
Project:	THARISA MINERALS	Date Tested:	31/05/2011	
Batch No.:	1039/F42/04/2011	Laboratory Number:	C121	
Field Sample Number:	TP 6	Depth (m):	4.7 - 5.1	

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

	, , ,	
Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.	

SATURATION DATA Test No. 2

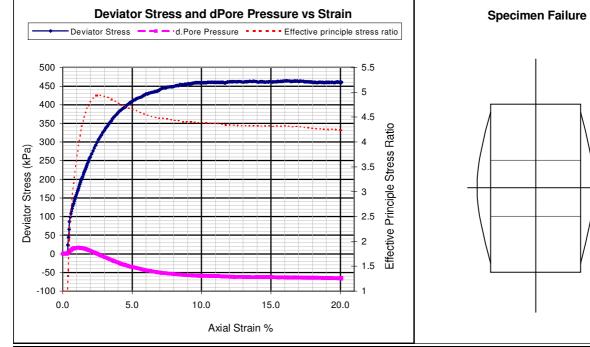
Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.98

CONSOLIDATION DATA

Effective cons. Stress (kPa	a):	76.2	t100) (minutes):	55	Side	drains fitted:	No
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	9.7	2.044	0.6479	51	3.368
CONSOLIDATED	99.13	49.57	1929.50	15.7	2.098	0.6051	87	3.366 Determined
FINAL (After shear)	79.25	55.44	2413.64	15.7	2.098	0.6054	87	Determined
Initial pore pressure (kPa): 341.0 Final pore pressure (kPa): 341.0 Pore pressure dissipation: 0%								
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour): 9					
Initial pore pressure (kPa): 363.3	Initial effective stress (kPa):	: 76.2			
Failure Criterion: Max. E	ffective Principle Stress Ratio				
Axial strain at failure (%):	2.59				
Deviator stress (kPa):	306.9		Principle St	resses (kPa)	
Excess pore pressure (kPa):	-1.6	σ 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	4.946	383.2	384.7	76.2	77.8



Civilab

Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	31/05/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C121
Field Sample Number:	TP 6	Depth (m):	4.7 - 5.1

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

	, , ,
Remarks:	A Consolidated Undrained test on a remoulded sample tested saturated.
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SATURATION DATA Test No. 3

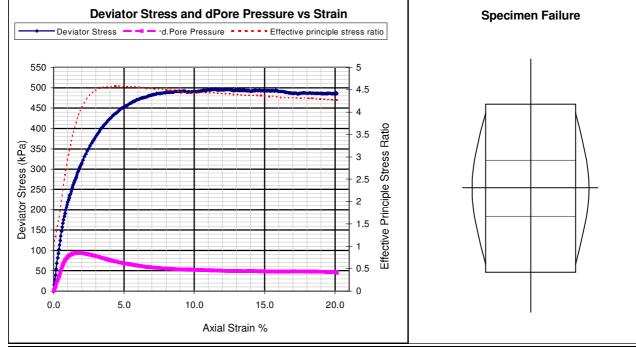
Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.99

CONSOLIDATION DATA

Effective cons. Stress (kPa	a):	194.9	t100) (minutes):	150	Side	drains fitted:	No
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	9.7	2.053	0.6401	51	3.368
CONSOLIDATED	98.44	49.21	1902.16	15.6	2.154	0.5633	93	0.300 Determined
FINAL (After shear)	78.65	55.06	2380.75	15.6	2.153	0.5641	93	Determined
Initial pore pressure (kPa): 344.9 Final pore pressure (kPa): 345.2 Pore pressure dissipation: -6%								
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

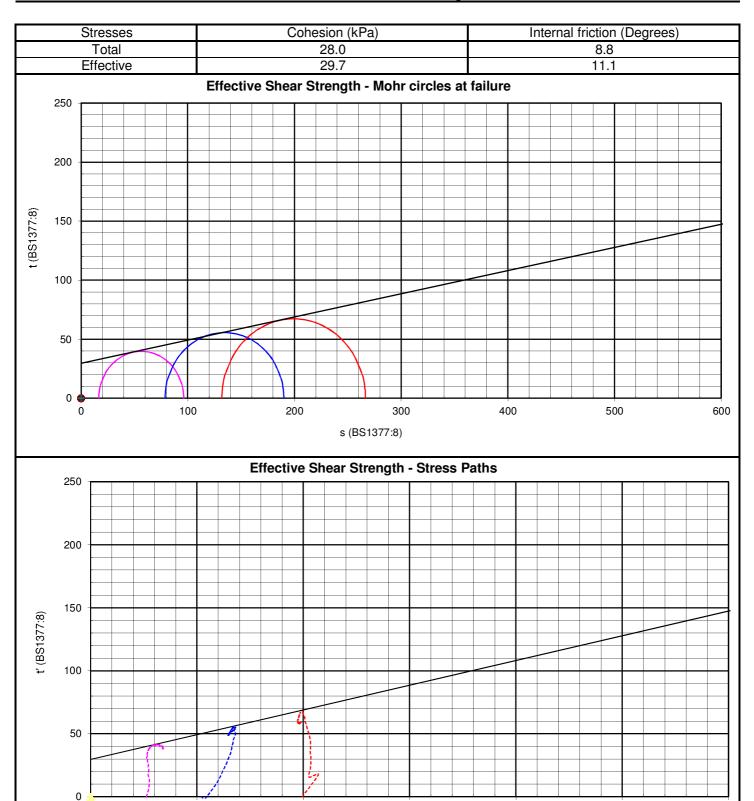
Rate of strain (%/hour): 3.48			
Initial pore pressure (kPa): 344.7	7 Initial effective stress (kPa): 194.9		
Failure Oritorian	ffective Driverints Others Datis		
Failure Criterion: Max. Ef	ffective Principle Stress Ratio		
Axial strain at failure (%):	4.72		
Deviator stress (kPa):	446.4 Principle Stre	esses (kPa)	
Excess pore pressure (kPa):	70.5	O 3	O 3'
Effective principle stress ratio:	4.589 641.2 570.8	194.9	124.4



Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	24/06/2011
Proj.No.:	F42/04/11	Laboratory Number:	C123
Field Sample Reference:	TP 10	Depth (m):	4.0-4.2

Effective Shear Strength



s' (BS1377:8)

300

400

500

600

200

100

Triaxial	Com	pression	Test	Results
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Project:	THARISA MINERALS	Date Tested:	24/06/2011
i iojeci.	THAI HOA WIINEI IAEO	Dale Testeu.	24/00/2011
Batch No.:	F42/04/11	Laboratory Number:	C123
Field Sample Number:	TP 10	Depth (m):	4.0-4.2

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated.

SATURATION DATA Test No. 1

Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.98

CONSOLIDATION DATA

Effective cons. Stress (kPa): 52.		52.4	2.4 t100 (minutes): 75			Side drains fitted: No		
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*100	*50	1963.50	38.4	1299	1.0405	98	0.05
CONSOLIDATED	99.64	49.82	1949.45	48.6	1313	1.0186	127	2.65 Assumed
FINAL (After shear)	92.62	51.67	2097.21	48.6	1313	1.0187	127	Assumed
Initial pore pressure (kPa):	tial pore pressure (kPa): 344.0 Final pore pressure (kPa): 344.0 Pore pressure dissipation: 0%					n: 0%		
*: Measured dimensions: all other dimensions are calculated.								

SHEAR DATA

7.6						
344.6	Initial effective s	stress (kPa): 5	2.4			
/lax. Effective P	Principle Stress Rati	0				
2.22	2					
79.6	6			Principle Str	esses (kPa)	1
: 35.7	7		σ 1	σ1'	O 3	O 3'
io: 5.75	59		132.0	96.3	52.4	16.7
	344.6 Max. Effective F 2.22 79.6 : 35.7	344.6 Initial effective s Max. Effective Principle Stress Ration 2.22 79.6 : 35.7	344.6 Initial effective stress (kPa): 5 Max. Effective Principle Stress Ratio 2.22 79.6 : 35.7	344.6 Initial effective stress (kPa): 52.4 Max. Effective Principle Stress Ratio 2.22 79.6 : 35.7 σ1	344.6 Initial effective stress (kPa): 52.4 Max. Effective Principle Stress Ratio 2.22 79.6 Principle Str : 35.7	344.6 Initial effective stress (kPa): 52.4 Max. Effective Principle Stress Ratio 2.22 79.6 Principle Stresses (kPa) : 35.7

Deviator stress corrections: Membrane correction: 1.1 kPa **Deviator Stress and dPore Pressure vs Strain Specimen Failure Deviator Stress** d.Pore Pressure --- Effective principle stress ratio 90 6 5.5 80 5 70 4.5 60 Deviator Stress (kPa) 50 40 3 30 2.5 20 2 10 1.5 0 2.0 4.0 6.0 8.0 0.0 Axial Strain %

Triaxial	Com	pression	Test	Results
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Project:	THARISA MINERALS	Date Tested:	24/06/2011
Batch No.:	F42/04/11	Laboratory Number:	C123
Field Sample Number:	TP 10	Depth (m):	4.0-4.2

Remarks:	A Consolidated Undrained test on an undisturbed sample tested saturated. Multistage
	Loading.

SATURATION DATA Test No. 2

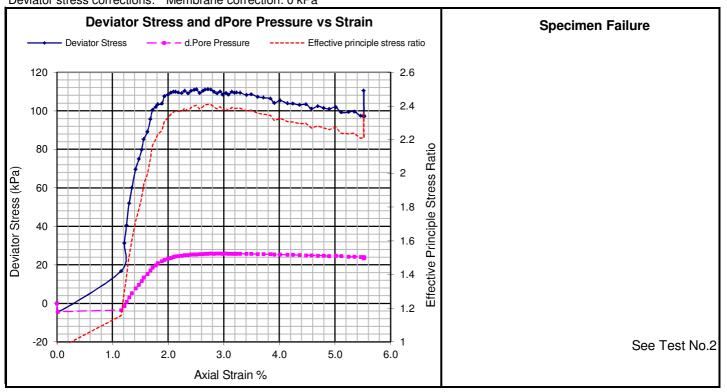
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.98

CONSOLIDATION DATA

Effective cons. Stress (kPa):		104.7 t100 (minutes): 250			Side drains fitted: No			
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*94.45	*51.67	2097.21	38.4	1287	1.0585	96	0.05
CONSOLIDATED	93.61	51.21	2059.91	48.6	1323	1.0036	128	2.65 Assumed
FINAL (After shear)	88.44	52.69	2180.31	48.6	1322	1.0040	128	Assumed
Initial pore pressure (kPa): 344.0 Final pore pressure (kPa): 344.0 Pore pressure dissipation: 0%					on: 0%			
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

_								
Rate of strain (%/hour):	2.3							
Initial pore pressure (kPa):	e pressure (kPa): 342.3 Initial effective stress (kPa): 104.7							
Parameters at failure:								
Failure Criterion:	Max. Deviator	Stress						
Axial strain (%):	2.7	'2						
Deviator stress (kPa):	Deviator stress (kPa): 111.2 Principle Stresses (kPa)						1	
Excess pore pressure (kPa)): 25.	.8			σ 1	σ1'	O 3	σ3'
Effective principle stress rat	io: 2.4	110			215.9	190.1	104.7	78.9



Triaxial	Compress	ion Test	Results
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Project:	THARISA MINERALS	Date Tested:	24/06/2011
Batch No.:	F42/04/11	Laboratory Number:	C123
Field Sample Number:	TP 10	Depth (m):	4.0-4.2

Remarks: A Consolidated Undrained test on an undisturbed sample tested saturated. Multistage Loading.

SATURATION DATA Test No. 3

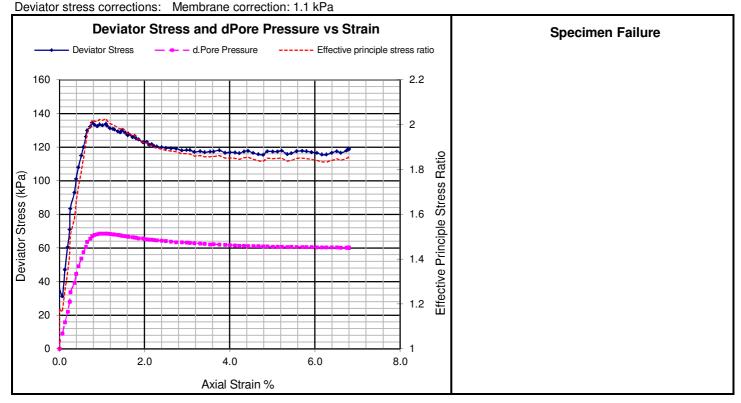
Saturation method: Alternating	increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 357.0	Final back pressure (kPa): 347.0	Final B parameter: 0.98

CONSOLIDATION DATA

Effective cons. Stress (kPa):		199.0	t10	0 (minutes):	750	Side drains fitted: No		
	Height	Diameter	Area	Moisture	Dry Density	Void	Saturation	Specific
	mm	mm	mm^2	Content %	kg/m³	Ratio	%	Gravity
INITIAL (Before saturation)	*87.61	*52.69	2180.31	38.4	1335	0.9851	103	0.05
CONSOLIDATED	86.92	52.27	2145.84	48.6	1367	0.9381	137	2.65 Assumed
FINAL (After shear)	81.02	54.14	2302.21	48.6	1367	0.9383	137	Assumed
Initial pore pressure (kPa): 345.0 Final pore pressure (kPa): 345.0			345.0	Pore pressi	ure dissipatio	n: 0%		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour):	0.8								
Initial pore pressure (kPa):	348.0	Initi	al effective str	ess (kPa):	199.0				
Parameters at failure:									
Failure Criterion:	Max. Devia	ator Stress							
Axial strain (%):		0.77							
Deviator stress (kPa):		134.6				Principle	Stre	sses (kPa)	
Excess pore pressure (kPa)):	66.9			σ1	σ1'		σ 3	O 3'
Effective principle stress rat	io:	2.019			333.5	266.	6	199.0	132.0
D. Salara principle attack			1 B		000.0				



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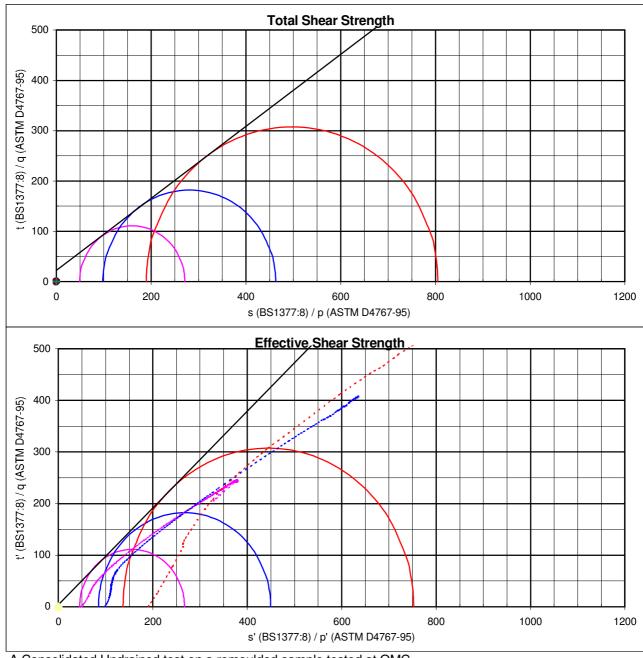
Civil Engineering Testing Laboratory

Triaxial Compression Test Results

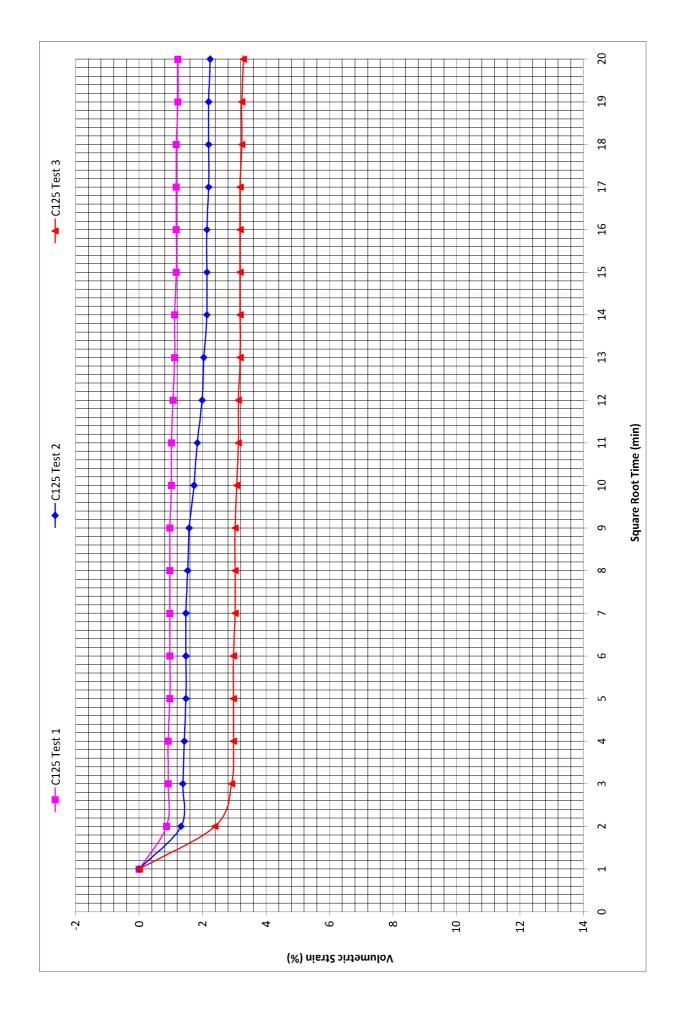
Project:	THARISA MINERALS	Date Tested:	31/05/2011
Proj.No.:	1039/F42/04/2011	Laboratory Number:	C125
Field Sample Reference:	TP 19	Depth (m):	1.2 - 3.4

Mohr Stress Circles

	COHESION (kPa)	FRICTION ANGLE
TOTAL STRESSES	22	36
EFFECTIVE STRESSES	3	43



A Consolidated Undrained test on a remoulded sample tested at OMC.



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Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	31/05/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C125
Field Sample Number:	TP 19	Depth (m):	1.2 - 3.4

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

	, , ,	
Remarks:	A Consolidated Undrained test on a remoulded sample tested at OMC.	

SATURATION DATA Test No. 1

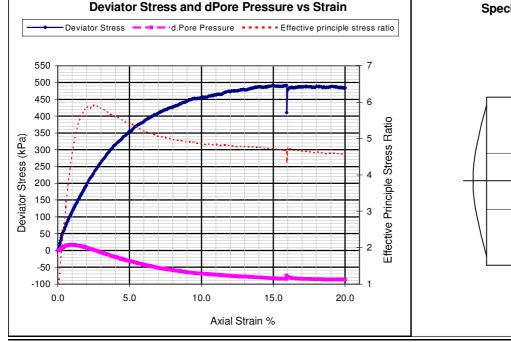
Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.96

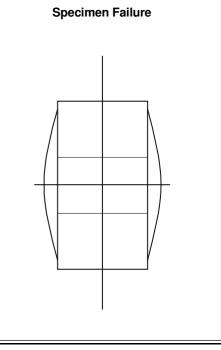
CONSOLIDATION DATA

Effective cons. Stress (kPa): 49.0			t100 (minutes): 38.4			Side drains fitted: No		
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	14.0	1.994	0.6430	72	3.276
CONSOLIDATED	99.58	49.79	1946.83	17.7	2.020	0.6221	93	3.276 Determined
FINAL (After shear)	79.68	55.66	2433.08	17.7	2.020	0.6222	93	Determined
Initial pore pressure (kPa): 382.9 Final pore pressure (kPa): 342.1 Pore pressure dissip					ure dissipatio	n: 95%		
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour): 9					
Initial pore pressure (kPa): 341.0	Initial effective stress (kPa):	: 49.0			
Failure Oritariana Mara Ff	taatina Bulmalula Otusaa Batia				
Failure Criterion: Max. Ef	fective Principle Stress Ratio				
Axial strain at failure (%):	2.37				
Deviator stress (kPa):	222.0		Principle St	resses (kPa))
Excess pore pressure (kPa):	3.9	σ 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	5.923	271.0	267.1	49.0	45.1







Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	31/05/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C125
Field Sample Number:	TP 19	Depth (m):	1.2 - 3.4

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested at OMC.	

SATURATION DATA Test No. 2

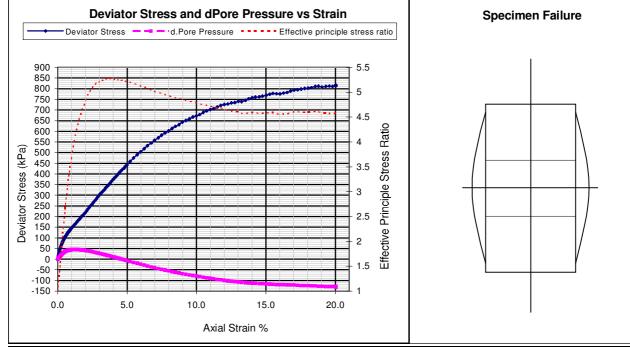
Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.97

CONSOLIDATION DATA

Effective cons. Stress (kPa	98.3	t100 (minutes): 50.4			Side drains fitted: No					
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific		
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity		
INITIAL (Before saturation)	* 100.00	* 50.00	1963.50	12.7	2.020	0.6215	67	3.276		
CONSOLIDATED	99.24	49.62	1933.50	15.5	2.068	0.5843	87	3.276 Determined		
FINAL (After shear)	79.36	55.48	2417.66	15.5	2.068	0.5845	87	Determined		
Initial pore pressure (kPa): 433.0 Fina			l pore pres	sure (kPa):	343.6	Pore pressu	ıre dissipatio	n: 96%		
*: Measured dimensions; a	*: Measured dimensions; all other dimensions are calculated.									

SHEAR DATA

Rate of strain (%/hour):	9					
Initial pore pressure (kPa): 3	341.5 lı	nitial effective stress (kPa):	98.3			
Failure Criterion: Ma	x. Effective P	rinciple Stress Ratio				
Axial strain at failure (%):	3.85					
Deviator stress (kPa):	364.5			Principle St	resses (kPa)	
Excess pore pressure (kPa):	13.2		σ 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	5.283		462.8	449.6	98.3	85.1



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Civil Engineering Testing Laboratory

Triaxial Compression Test Results

Project:	THARISA MINERALS	Date Tested:	31/05/2011
Batch No.:	1039/F42/04/2011	Laboratory Number:	C125
Field Sample Number:	TP 19	Depth (m):	1.2 - 3.4

This test was carried out in accordance with BS 1377:Part 8:1990 Clause 4,5,6,7

Remarks:	A Consolidated Undrained test on a remoulded sample tested at OMC.	

SATURATION DATA Test No. 3

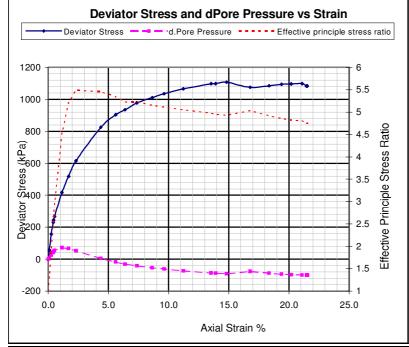
Saturation method: Alternatin	g increments of cell- & back pressure	
Pressure increments applied (kPa):	50,70,100,100,100	Differential pressure (kPa): 10.0
Final cell pressure (kPa): 350.0	Final back pressure (kPa): 340.0	Final B parameter: 0.98

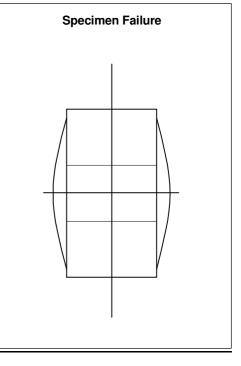
CONSOLIDATION DATA

Effective cons. Stress (kPa	189.7	89.7 t100 (minutes): 4.41			Side drains fitted: No			
	Height	Diameter	Area	Moisture	Dry Unit	Void	Saturation	Specific
	mm	mm	mm ²	Content %	Weight	Ratio	%	Gravity
INITIAL (Before saturation)	* 96.00	* 50.00	1963.50	14.5	2.075	0.5789	82	3.276
CONSOLIDATED	94.83	49.39	1915.58	15.0	2.154	0.5211	94	3.276 Determined
FINAL (After shear)	74.43	55.74	2440.62	15.0	2.153	0.5216	94	Determined
Initial pore pressure (kPa): 537.5 Final			l pore pres	sure (kPa):	345.1	Pore pressu	ure dissipatio	n: 97%
*: Measured dimensions; all other dimensions are calculated.								

SHEAR DATA

Rate of strain (%/hour): 9					
Initial pore pressure (kPa): 350	1.3 Initial effective stres	ss (kPa): 189.7			
Failure Criterion: Max.	Effective Principle Stress Ra	atio			
Axial strain at failure (%):	2.32				
Deviator stress (kPa):	615.4		Principle St	resses (kPa)	
Excess pore pressure (kPa):	52.7	σ 1	σ 1'	O 3	O 3'
Effective principle stress ratio:	5.492	805.1	752.4	189.7	137.0





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Civil Engineering Testing Laboratories

Flexible Wall Constant Head Permeability Test Results

PROJECT: THARISA MINERALS DATE: 31/05/2011
PROJECT No.: F42/04/2011

Sample	Moisture Content		Dry	Co-efficient of		
Depth	(%)		Density	Permeability (m/s)		s)
in	Before	After	(Kg/m ³)	Ran	ge	
metres	Test	Test		Minimum	Maximum	Average
1.6 - 4.0	14.1	20.2	1830	3.8E-06	5.5E-06	4.5E-06
1.0 - 1.2	33.7	37.0	1259	7.6E-10	1.1E-09	9.1E-10
1.0 - 1.2	36.4	36.6	1298	1.0E-10	7.8E-10	4.7E-10
4.7 - 5.1	10.6	14.5	1933	1.0E-07	6.2E-07	3.0E-07
4.5 - 4.7	38.2	48.4	1222	6.5E-10	1.9E-09	1.0E-09
4.0 - 4.2	45.2	46.3	1121	2.2E-10	1.1E-09	5.4E-10
Undisturbed	samples	•	•			
Effective ce	ll pressure	100kPa.				
	Depth in metres 1.6 - 4.0 1.0 - 1.2 1.0 - 1.2 4.7 - 5.1 4.5 - 4.7 4.0 - 4.2 Undisturbed Effective ce	Depth in metres (%) Before Test 1.6 - 4.0 14.1 1.0 - 1.2 33.7 1.0 - 1.2 36.4 4.7 - 5.1 10.6 4.5 - 4.7 38.2 4.0 - 4.2 45.2 Undisturbed samples	Depth in metres (%) Eefore Test After Test 1.6 - 4.0 14.1 20.2 1.0 - 1.2 33.7 37.0 1.0 - 1.2 36.4 36.6 4.7 - 5.1 10.6 14.5 4.5 - 4.7 38.2 48.4 4.0 - 4.2 45.2 46.3 Undisturbed samples Effective cell pressure 100kPa.	Depth in metres (%) Effective cell pressure Density (Kg/m³) 1.6 - 4.0 14.1 20.2 1830 1.0 - 1.2 33.7 37.0 1259 1.0 - 1.2 36.4 36.6 1298 4.7 - 5.1 10.6 14.5 1933 4.5 - 4.7 38.2 48.4 1222 4.0 - 4.2 45.2 46.3 1121 Undisturbed samples Effective cell pressure 100kPa.	Depth in metres (%) Test Density (Kg/m³) Perm Ran Minimum 1.6 - 4.0 14.1 20.2 1830 3.8E-06 1.0 - 1.2 33.7 37.0 1259 7.6E-10 1.0 - 1.2 36.4 36.6 1298 1.0E-10 4.7 - 5.1 10.6 14.5 1933 1.0E-07 4.5 - 4.7 38.2 48.4 1222 6.5E-10 4.0 - 4.2 45.2 46.3 1121 2.2E-10 Undisturbed samples Effective cell pressure 100kPa.	Depth in metres (%) East Density (Kg/m³) Permeability (m/s Range Minimum Maximum 1.6 - 4.0 14.1 20.2 1830 3.8E-06 5.5E-06 1.0 - 1.2 33.7 37.0 1259 7.6E-10 1.1E-09 1.0 - 1.2 36.4 36.6 1298 1.0E-10 7.8E-10 4.7 - 5.1 10.6 14.5 1933 1.0E-07 6.2E-07 4.5 - 4.7 38.2 48.4 1222 6.5E-10 1.9E-09 4.0 - 4.2 45.2 46.3 1121 2.2E-10 1.1E-09 Undisturbed samples Effective cell pressure 100kPa.

Pressure Difference = 20 kPa

Investment Facility Company 842 (Pty) Limited trading as Civilab. Registration No: 98/19071/07

BRANCHES: CENTURION

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Civil Engineering Testing Laboratories

Flexible Wall Constant Head Permeability Test Results

PROJECT:	THARISA MINERALS	DATE : 31/05/2011
PROJECT No.:	F42/04/2011	

Field Sample	Sample	Moisture Content		Dry	Co-efficient of			
	Depth	(%)	_	Density	Permeability (m/s)			
Number	in	Before	After	(Kg/m³)	Ran	ge		
	metres	Test	Test		Minimum	Maximum	Average	
TP 19 (C125)	1.2 - 3.4	16.9	18.9	2209	2.6E-06	2.8E-06	2.7E-06	
DEMARKS :								

REMARKS: Undisturbed samples

Effective cell pressure 100kPa. Pressure Difference = 20 kPa

Investment Facility Company 842 (Pty) Limited trading as Civilab. Registration No: 98/19071/07

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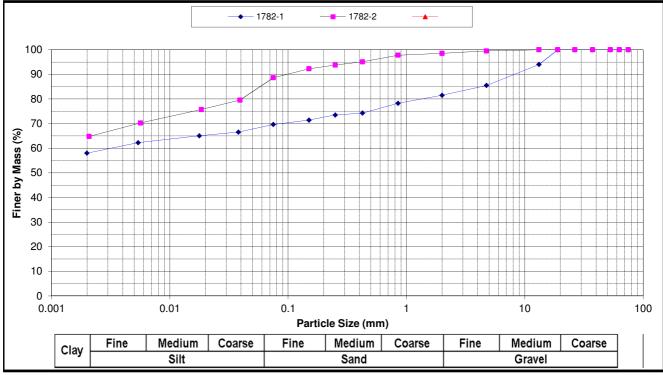
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Civil Engineering Testing Laboratories

Foundation Indicator Test Data

Project	THARISA MINERALS								
Project No.	2013-B-1782	2		Date	3 December	2013			
Sample No.	1782-1	1782-2		Sample No.	1782-1	1782-2			
Field Ref. No.	TP1	TP2		%Gravel	19	1			
Depth	1.7	1.8		%Sand	13	13			
Sieve size	%Passing	% Passing	% Passing	%Silt	11	21			
75	100	100		%Clay	58	65			
63	100	100		NMC %	Not Tested	Not Tested			
53	100	100		Liquid Limit	86	74			
37.5	100	100		Plasticity	56	42			
26.5	100	100		Index	30	72			
19.0	100	100		Linear Shrink.	20.5	20.			
13.2	94	100		Overall P.I.	42	40			
4.75	86	100		Grading	0.75	0.18			
2.00	82	99		Modulus	0.73	0.10			
0.85	78	98		H.R.B.	A-7-6 (18)	A-7-5 (20)			
0.425	74	95		Unified	CH	CH			
0.250	74	94		Weston swell					
0.150	71	92		(%) at 1 kPa					
0.075	70	89		Analysis as p	er method D4	22 of ASTM o	of 1985		
0.04	67	80		The results reported relate only to the					
0.02	65	76		samples tested.					
0.006	63	71		Documents may only be reproduced or					
0.002	58	65		published in t	heir full conte	xt.			



Remarks:

VRYHEID